



## HARDY FRAMES Inc. A MiTek Company

ardy Frames, Inc manufactures and markets pre-fabricated shear wall systems. We have been the innovative leader in the industry for over 15 years. Our company first introduced the Original Hardy Frame<sup>®</sup> Panels and Brace Frames in 1996. In 2006, we joined forces with SidePlate Systems, Inc. to launch the Hardy Frame<sup>®</sup> Special Moment Frame, the first pre-engineered, pre-fabricated Moment Frame in the industry.

The Hardy Frame<sup>®</sup> Moment Frame may be used in single and multistory structures, and uses the new-generation SidePlate<sup>®</sup> moment connection to resist lateral forces from earthquakes and wind. This connection was conceived and developed by David Houghton, a licensed structural engineer, to eliminate all problems that were exposed in traditional moment connections as a result of the 1994 Northridge earthquake. The outstanding performance of the SidePlate<sup>®</sup> system is nationally recognized and is the same technology we use in the Hardy Frame<sup>®</sup> Moment Frame to provide high allowable loads in narrow wall sections. Adding to the structural benefits, our Moment Frame is designed to arrive at the jobsite completely pre-assembled with no field welding and no special inspection required.

The mission of Hardy Frames is to provide cost effective, state-of-the-art, lateral force resisting systems that are easily installed to meet architectural, engineering and code requirements.

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### HARDYATE FRAMES

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### **General Information**

### **Code Evaluations:**

SidePlate<sup>®</sup> has been evaluated by the following code authorities: ICC Evaluation Service, Inc.: ICC-ES ESR-1275
The City of Los Angeles, California: Research Report - RR 25393

### Testing:

The SidePlate<sup>®</sup> connections have been extensively tested and qualified in accordance with AISC and ICC criteria. Products in this catalog are designed in accordance with the provisions of ICC-ES ESR-1275. A calculation package, stamped and signed by a registered design professional of SidePlate Systems, Inc., is provided for building department submittal which includes the complete design of the moment frame conforming to SMF standards.

### **Product Use:**

The Hardy Frame<sup>®</sup> products are designed and manufactured for the specific purposes described in this catalog. Any changes to the products or in the installation procedures must be approved by the designer of record and are the sole responsibility of the designer.

### **Quality Statement:**

Hardy Frames, Inc. warrants to its customers that its products are free from material defects of manufacture or design, and will perform in substantial accordance with published specifications, if properly used.

Hardy Frame<sup>®</sup> Moment Frames are manufactured from prime quality steel:

### **Plate Material:**

- ASTM A572 Grade 50: SidePlate and built up columns
- ASTM A36: Base Plates

### **Rolled Shapes:**

- ASTM A992 (F<sub>v</sub>=50ksi): Columns (W-sections)
- ASTM A500 Grade B (F<sub>y</sub>=46ksi): Beams (Hollow Structural Sections - HSS)

### Misc. Materials

- ASTM A36/F1554-36: Anchor bolts at column base plates
- ASTM A36: All other fasteners and components
- Nelson Studs: F<sub>v</sub>=70 ksi
- No. 2 Douglas Fir Larch or better: Beam and column nailers

Hardy Frames, Inc. reserves the right to change specifications, designs, and models without notice and liability of such changes.

The information presented in this catalog supersedes all information published in previous documents and publications.

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## **Moment Frame Design and Construction Standards**

### **Referenced Documents:**

- American Institute of Steel Construction Specification for Structural Steel Buildings (AISC 360)
- American Institute of Steel Construction Seismic Provisions for Structural Steel Buildings Part I and Part III, Supplement No. 1 dated Nov. 16, 2005 (AISC 341s1-05)
- American Welding Society D1.1 2006 Structural Welding Code-Steel (AWS D1.1)
- International Code Council-Evaluation Service Engineering Report ESR-1275
- City of Los Angeles Research Report 25393
- Los Angeles Building Code-2008 & 2010
- International Building Code-2006 & 2009
- LADBS Information Bulletin/Public-Building Code P/BC 2002-098 rev 3-09-2010

### **Frame Design Input:**

Frame Modeling Software: ETABS

Model represents P-Delta effects, fully rigid panel zone & SidePlate<sup>®</sup> connection stiffness and pinned column bases

Allowable Story Drift,  $\Delta_a$ : All moment frames were designed using  $\Delta_a = 0.025h_{sx}$ . **Load Cases:** Dead (DL, includes self-weight), Live (LL), Roof Live (LL,), Snow (S), Earthquake (EQ) and Wind (W)

### **Load Combinations:**

1.4DLa

1.2DL<sup>a</sup>+1.6LL+0.5(LL<sub>r</sub> or S)

1.2DL+1.6(LL<sub>r</sub> or S)+(0.5LL or 0.8W)

1.2DL+1.6W+0.5LL+(0.5LL<sub>r</sub> or 0.5S)

1.2DL+1.0EQ<sup>b</sup>+0.5LL+0.7S

0.9DL+1.6W

0.9DL+1.0EQb

0.9DL-1.0EQb

a. Notational load equal to 0.2% of the dead load included.

b. Includes assumed values of  $\rho{=}1.0$  and  $\ensuremath{\text{S}_{ds}}{=}2.0.$ 

### **Design Factors of Safety:**

Stress and vertical beam deflections are limited to 90% of code allowable for all designs. Under 90% limit for stress, stress governs over L/240 and L/360 beam deflection criteria; for actual reported deflections see "Hardy Frame<sup>®</sup> Moment Frame Calculations" design package.

### SidePlate® Connection Design:

The SidePlate<sup>®</sup> connection design used to complete the Hardy Frame<sup>®</sup> Moment Frame design is prepared by a registered design professional of SidePlate Systems, Inc. using the SidePlate<sup>®</sup> Connection Design Software (compliant with the requirements of ICC-ES ESR - 1275 & LA RR#25393). A strong column-weak beam relationship at the SidePlate<sup>®</sup> connection is achieved for all moment frame designs.



### **Designing with Hardy Frame® Moment Frame Overview**

### **When to use a Hardy Frame Moment Frame:**

- At large openings where the narrowest wall space is available and high shear loads are required
- When foundations are limited or existing and a pinned base connection facilitates foundation design
- When designing three story or greater structures, all parallel walls must be designed at the lowest R-Value. Specify a Hardy Frame Moment Frame and design at the same R=6.5 as design of plywood shear walls.
- NOTE: Standard Hardy Frame Moment Frames are listed for opening widths up to 23 feet and wall heights up to 20 feet. When both the width and the height exceed 13 feet, the Frame is over-sized for trucking and must ship as a "Knock-Down" unit, which requires field welding and special inspection to be supplied by others.

### Design:

- Calculate the design shear load based on an R-Value of 6.5
- Select a Hardy Frame Moment Frame based on the required geometry
- Check that the tabulated Allowable Shear meets the design shear load
- If the design shear load exceeds the allowable table value, contact Hardy Frames to determine if an alternate solution, such as a "fixed base" connection will meet the design loads.
- Calculate the additional vertical loads using the IBC strength design load combinations
- Check that additional vertical loads do not exceed the tabulated Allowable Vertical Loads
- Calculate the tabulated Column Base Reactions for use in the foundation design. For more information see the "Shear Multiplier" column in the Allowable Load tables and the corresponding table notes.

### **Specification:**

### On the Foundation Plan

- Specify the Hardy Frame Moment Frame model number at the location to be installed
- Specify anchorage for tension and shear from Hardy Frame Moment Frame Anchorage Solutions
- Specify connection of column base to the foundation (on Nuts & Washers, on Concrete, with Pinned or Fixed Base) refer to Typical Installation
   Details for standard connections

### On the Framing Plan

- Specify a connection from the collector (or "drag") to the top of the Hardy Frame Moment Frame. Refer to Typical Installation Details for standard connections
- Consider out of plane bracing for the Frame either through a diaphragm to the beam or with framing members to the columns

### **Submittal:**

- Include a calculation package for the model number specified (provided by Hardy Frames)
- Include Typical Installation Detail Sheets HFMF1 and HFMF2 as "supplemental pages"

### **Additional Items:**

- The table values in this catalog for Allowable Shear Loads assume Pinned Base anchorage.
- When additional shear load or a drift limit less than the code maximum is needed a Fixed Base design is available. Consult with Hardy Frames to determine fixed base values and options.
- All allowable shear loads have been adjusted to an R-Value of 6.5. To convert design loads from R=3.5 to R=6.5, multiply the design load by 3.5/6.5.
- The allowable shear loads are based on a story drift of 0.025h. To convert shear loads to a story drift of 0.020h multiply the table value by a factor of 0.8.
- When designing lateral resistance for a wall line with varying shear resisting elements the stiffness (drift/allowable shear load) must be proportioned.
- Two story Moment Frames are available. Fill out the Non-Standard Worksheet at the back of this catalog and submit to Hardy Frame to check the required configuration and loading.

### **Design Example**

9' 1" Nominal Wall Height Given:

16' x 7' Garage Door Opening

6 inch concrete curb at garage wall framing

21 foot wall length

ASD Seismic Design Shear  $(V_{DSN})$  at R of 6.5 = 16,000 lbs

No overturning loads applied from above

Uniform Loading on Header: DL = 256 lbs/LF

LL = 480 lbs/LF

### **Look Up Model Numbers by Width**

From Table 1.0 – Standard Widths:

- HF-D9 16x: Win = 16' 7" Wout = 18' 4"
- HF-D18 16x: Win = 16' 7" Wout = 19' 9-1/2" All Dimensions are compatible with Design Requirements
- HF-D18H 16x: Win = 16' 7" Wout = 19' 10"

### **Check Allowable Shear (V) and Opening Height (h1)**

From Table 1.9 - Nominal 9' 1" Wall Heights - Pinned Base:

HF-D18H 16x9: Allowable ASD Seismic Shear = 18,442 lbs > 16,000 lbs **OK** 

7' 9-1/4" Opening height (h1) + 6" concrete curb height = 8' 3-1/4" > 7' **OK** 

### Calculate and check the aggregate additional vertical load P<sub>Dsn</sub> for HF-D18H 16x9

From Table 1.9 - Nominal 9' 1" Wall Heights - Pinned Base:

- ullet Maximum Additional Vertical Load  $P_u = 35,280$  lbs
- Design DL = 256 lbs/LF x 19' 10" = 5,077 lbs
- Design LL = 480 lbs/LF x 19' 10" = 9,520 lbs
- E = Overturning Forces on Moment Frame header from Earthquake = 0
- W = Overturning Forces on Moment Frame header from Wind = 0
- $\Omega_0 = IBC$  Over-strength Factor

 $1.4DL = 1.4 \times 5,077 \text{ lbs} = 7,108 \text{ lbs}$ 

 $1.2DL + 1.6LL = (1.2 \times 5,077 \text{ lbs}) + (1.6 \times 9,520 \text{ lbs}) = 21,324 \text{ lbs}$  - Controls

 $1.2DL + 0.5LL + \Omega_0E = (1.2 \times 5,077 \text{ lbs}) + (0.5 \times 9,520 \text{ lbs} + 0) = 10,850 \text{ lbs}$ 

 $1.2DL + 0.5LL + 1.6W = (1.2 \times 5,077 \text{ lbs}) + (0.5 \times 9,520 \text{ lbs} + 0) = 10,850 \text{ lbs}$ 

 $0.9DL + 1.6W = (0.9 \times 5,077 \text{ lbs}) + 0 = 4,569 \text{ lbs}$ 

 $0.9DL + 1.0E = (0.9 \times 5,077 \text{ lbs}) + 0 = 4,569 \text{ lbs}$ 

 $\frac{Maximum\ P_{\underline{Dsn}}}{Minimum\ P_{\underline{Dsn}}} = 21,324\ lbs\ <\ 35,280\ lbs\ \mbox{OK}$   $\underline{Minimum\ P_{\underline{Dsn}}} = 4,569\ lbs$ 

### **USE - HF-D18H 16x9 with Pinned Base Connection**

### **SPECIFY ON FOUNDATION PLAN**

- Hardy Frame Moment Frame Model Number HF-D18H 16x9
- Anchorage = D18H-Pinned (See Anchorage Table in this Catalog)
- Bottom Connection = Installation at Concrete Curb on Nuts & Washers

### **SPECIFY ON FRAMING PLAN**

- Hardy Frame Moment Frame Model Number HF-D18H 16x9
- Top connection including quantity and spacing of USP MP4F connectors for shear transfer (see detail 5 of this catalog)

### FOR FOUNDATION DESIGN

### Calculate and check the column base resultant shear for HF-D18H 16x9

- $\bullet$  From Table 1.9 Shear Multiplier at compression column  $V_{Mult} = 0.342$
- $\bullet$  From Table 1.9 Maximum Resultant Shear at Column Base  $V_R = 16,548$  lbs

 $V_{B} = (V_{Dsn} / 2) + (V_{Mult} \times Maximum P_{Dsn}) = (16,000 lbs/ 2) + (0.342 \times 21,324 lbs) = 15,293 lbs$ 

### Calculate the column uplift for HF-D18H 16x9

- From Table 1.9 Wall Height = 9' 0-3/4'
- From Table 1.0 W<sub>CI</sub> = 18' 1"

 $Uplift = [(V_{Dsn} \text{ x MF Height}) / W_{CL}^{2}] - (Minimum P_{Dsn} / 2) = [(16,000 \text{ lbs x } 9.06 \text{ ft}) / 18.08 \text{ ft}] - (4,569 \text{ lbs } / 2) = 5,733 \text{ lbs } / 2) = 1,000 \text{ lbs } / 2 = 1,0$ 

### HARDY AT FRAMES

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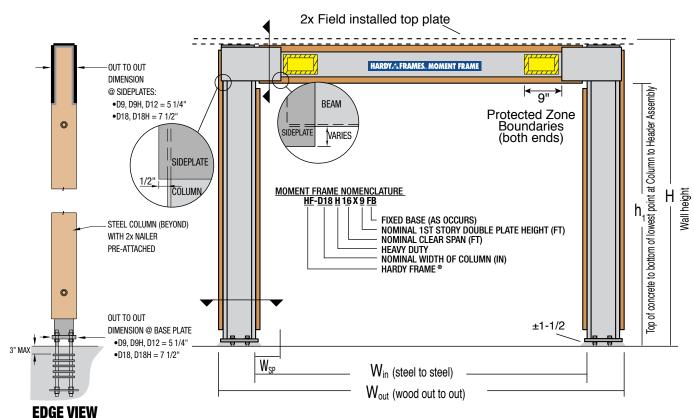






### HARDY A FRAMES

### **Column to Header Assembly Column Footprint Dimensions Column to Header Dimensions** 13 **D9 & D9H = BUILT UP COLUMN** TOP OF NAILER TO BOTTOM OF NAILER SIDEPLATE DIMENSION OF NAILER TO LOWEST OF HEADER ASSEMBLY 12 13 10-1/4 11-1/2 12-1/2 4" D12=W12 x 22 -5-1/4--5-1/4 OUT TO OUT D9 Col to 6x4 Hdr D9 Col to 7x4 Hdr D9 Col to 8x4 Hdr AT SIDEPLATE D12 Col to 6x4 Hdr $W_{SP} = 9-1/4$ $W_{SP} = 10$ $W_{\mbox{\scriptsize SP}} = SidePlate$ extension beyond $W_{SP} = 8-1/2$ inside column flange 4" 12-1/4" 13-1/4" 17-1/2 D18 = W18 x 35 11-1/2 13-1/2 6" 11 17-3/4" 18-3/4" D18H = W18x46 7-1/2 7-1/2 7-1/2 D18 Col to 6x6 Hdr D18 Col to 8x6 Hdr D18H Col to 10x6 Hdr $W_{SP} = 11$ $W_{SP} = 13$ $W_{SP} = 15$ 18" 18-3/4"



### HARDY A FRAMES IN

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	Table 1.0 Hardy Frame Moment Frame Width Dimensioning												
Nominal Opening Width (ft)	Model Number	W <sub>in</sub> Steel to Steel @ Column Flanges (ft-in)	W <sub>CL</sub> Centerline to Centerline @ Columns (ft-in)	W <sub>out-SP</sub> Out to Out @ SidePlate Connection (ft-in)	W <sub>out</sub> at Wood Nailers on Outside Column Flanges (ft-in)	Col-Beam Designation							
6	HF-D9 6x	6' 7"	7' 4"	8' 2"	8' 4"	D9-6x4							
7	HF-D9 7x	7' 7"	8' 4"	9' 2"	9' 4"	D9-6x4							
	HF-D9 8x		9' 4"	10' 2"	10' 4"	D9-6x4							
	HF-D9 <b>H</b> 8x	0, 7,,	9' 4"	10' 2"	10' 4"	D9-7x4							
8	HF-D12 8x	8' 7"	9' 7-1/4"	10' 8-1/2"	10' 10-1/2"	D12-6x4							
	HF-D18 8x		10' 0-3/4"	11' 7-1/2"	11' 9-1/2"	D18-6x6							
	HF-D9 9x		10' 4"	11' 2"	11' 4"	D9-7x4							
9	HF-D12 9x	9' 7"	10' 7-1/4"	11' 8-1/2"	11' 10-1/2"	D12-6x4							
	HF-D18 9x		11' 0-3/4"	12' 7-1/2"	12' 9-1/2"	D18-8x6							
	HF-D9 10x		11' 4"	12' 2"	12' 4"	D9-7x4							
10	HF-D12 10x	10' 7"	11' 7-1/4"	12' 8-1/2"	12' 10-1/2"	D12-6x4							
	HF-D18 10x		12' 0-3/4"	13' 7-1/2"	13' 9-1/2"	D18-8x6							
	HF-D9 11x		12' 4"	13' 2"	13' 4"	D9-7x4							
11	HF-D12 11x	11' 7"	12' 7-1/4"	13' 8-1/2"	13' 10-1/2"	D12-6x4							
	HF-D18 11x		13' 0-3/4"	14' 7-1/2"	14' 9-1/2"	D18-8x6							
	HF-D9 12x		13' 4"	14' 2"	14' 4"	D9-7x4							
12	HF-D12 12x	12' 7"	13' 7-1/4"	14' 8-1/2"	14' 10-1/2"	D12-6x4							
	HF-D18 12x		14' 0-3/4"	15' 7-1/2"	15' 9-1/2"	D18-8x6							
	HF-D9 13x		14' 4"	15' 2"	15' 4"	D9-7x4							
13	HF-D12 13x	13' 7"	14' 7-1/4"	15' 8-1/2"	15' 10-1/2"	D12-6x4							
	HF-D18 13x		15' 0-3/4"	16' 7-1/2"	16' 9-1/2"	D18-8x6							
	HF-D9 14x		15' 4"	16' 2"	16' 4"	D9-7x4							
44	HF-D12 14x	14, 7,	15' 7-1/4"	16' 8-1/2"	16' 10-1/2"	D12-6x4							
14	HF-D18 14x	14' 7"	16' 0-3/4"	17' 7-1/2"	17' 9-1/2"	D18-8x6							
	HF-D18 <b>H</b> 14x		16' 1"	17' 8"	17' 10"	D18-10x6							
	HF-D9 15x		16' 4"	17' 2"	17' 4"	D9-8x4							
15	HF-D18 15x	15' 7"	17' 0-3/4"	18' 7-1/2"	18' 9-1/2"	D18-8x6							
	HF-D18 <b>H</b> 15x		17' 1"	18' 8"	18' 10"	D18-10x6							
	HF-D9 16x		17' 4"	18' 2"	18' 4"	D9-8x4							
16	HF-D18 16x	16' 7"	18' 0-3/4"	19' 7-1/2"	19' 9-1/2"	D18-8x6							
	HF-D18 <b>H</b> 16x		18' 1"	19' 8"	19' 10"	D18-10x6							
17	HF-D18 <b>H</b> 17x	17' 7"	19' 1"	20' 8"	20' 10"	D18-10x6							
18	HF-D18 <b>H</b> 18x	18' 7"	20' 1"	21' 8"	21' 10"	D18-10x6							
19	HF-D18 <b>H</b> 19x	19' 7"	21' 1"	22' 8"	22' 10"	D18-10x6							
20	HF-D18 <b>H</b> 20x	20' 7"	22' 1"	23' 8"	23' 10"	D18-10x6							
21	HF-D18 <b>H</b> 21x	21' 7"	23' 1"	24' 8"	24' 10"	D18-10x6							
22	HF-D18 <b>H</b> 22x	22' 7"	24' 1"	25' 8"	25' 10"	D18-10x6							
23	HF-D18 <b>H</b> 23x	23' 7"	25' 1"	26' 8"	26' 10"	D18-10x6							

For two story moment frames see the non-standard worksheet at the back of this catalog

MODEL NUMBERING

HF-D18H 16 X 9 FB

"FB" = FIXED BASE (THIS OPTION REQUIRES A CUSTOM DESIGN)
NOMINAL DOUBLE PLATE HEIGHT (FT)
NOMINAL CLEAR SPAN (FT)
H = HEAVY DUTY
NOMINAL COLUMN WIDTH (IN)

HARDY FRAME®

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	Table 1.8 - 8' 1" Nominal Wall Heights Net Wall Ht = 8' 0-3/4"									
	Seism	ic	W	ind	Maximum Add'l	Shear Multiplier <sup>4</sup>	Column Base	Reactions	h1 <sup>7</sup>	Approx
Model	Allowable <sup>1</sup> Shear V @ R= 6.5 (lbs)	Drift <sup>2</sup> @ Allow V (in)	Allowable Shear V (lbs)	Drift <sup>2</sup> @ Allow V (in)	V4:13	@ Compression Column V <sub>Mult</sub>	Resultant Shear <sup>5</sup> V <sub>R</sub> @ Allow V & Max Pu (lbs)	Uplift <sup>6</sup> @ Allow V when Pu = 0 (lbs)	Opening Height (ft-in)	Weight (lbs)
HF-D9 6x8	7,569	0.410	10,597	0.574	51,800	0.149	8,470	7,982	7' 0-1/4"	987
HF-D9 7x8	6,684	0.410	9,358	0.574	45,360	0.169	8,008	6,201	7' 0-1/4"	1,019
HF-D9 8x8	5,956	0.410	8,338	0.574	40,320	0.189	7,610	4,935	7' 0-1/4"	1,051
HF-D9 <b>H</b> 8x8	7,789	0.410	10,905	0.574	49,840	0.182	9,387	6,455	6' 11-1/4"	1,102
HF-D12 8x8	6,392	0.410	8,949	0.574	40,600	0.201	8,151	5,143	7' 0-1/4"	890
HF-D18 8x8	13,130	0.410	14,706	0.459	18,144	0.236	9,167	10,085	6' 11-3/4"	1,412
HF-D9 9x8	7,102	0.410	9,943	0.574	44,800	0.202	9,046	5,317	6' 11-1/4"	1,137
HF-D12 9x8	5,722	0.410	8,011	0.574	36,400	0.222	7,756	4,172	7' 0-1/4"	922
HF-D18 9x8	20,386	0.410	22,832	0.459	23,520	0.250	13,763	14,243	6' 9-1/4"	1,602
HF-D9 10x8	6,512	0.410	9,117	0.574	40,880	0.222	8,771	4,446	6' 11-1/4"	1,173
HF-D12 10x8	5,166	0.410	7,232	0.574	33,040	0.241	7,425	3,442	7' 0-1/4"	954
HF-D18 10x8	18,218	0.410	20,404	0.459	35,280	0.269	14,869	11,679	6' 9-1/4"	1,650
HF-D9 11x8	6,004	0.410	8,406	0.574	37,520	0.242	8,517	3,763	6' 11-1/4"	1,208
HF-D12 11x8	4,699	0.410	6,579	0.574	29,960	0.262	7,110	2,886	7' 0-1/4"	986
HF-D18 11x8	16,418	0.410	18,388	0.459	39,480	0.289	15,129	9,717	6' 9-1/4"	1,698
HF-D9 12x8	5,564	0.410	7,790	0.574	34,440	0.262	8,266	3,228	6' 11-1/4"	1,244
HF-D12 12x8	4,304	0.410	6,026	0.574	27,440	0.282	6,856	2,444	7' 0-1/4"	1,018
HF-D18 12x8	14,901	0.410	16,689	0.459	25,200	0.311	12,206	8,190	6' 9-1/4"	1,746
HF-D9 13x8	5,180	0.410	7,252	0.574	31,920	0.282	8,062	2,792	6' 11-1/4"	1,280
HF-D12 13x8	3,967	0.410	5,554	0.574	25,480	0.302	6,659	2,103	7' 0-1/4"	1,050
HF-D18 13x8	13,616	0.410	15,250	0.459	29,400	0.330	12,697	6,991	6' 9-1/4"	1,794
HF-D9 14x8	4,842	0.410	6,779	0.574	29,680	0.303	7,884	2,444	6' 11-1/4"	1,315
HF-D12 14x8	3,676	0.410	5,146	0.574	23,520	0.322	6,442	1,821	7' 0-1/4"	1,083
HF-D18 14x8	12,513	0.410	14,015	0.459	31,080	0.350	12,856	6,020	6' 9-1/4"	1,842
HF-D18 <b>H</b> 14x8	25,027	0.410	28,030	0.459	34,272	0.351	19,808	12,027	6' 5-3/4"	2,372
HF-D9 15x8	5,914	0.410	8,280	0.574	33,600	0.314	9,360	2,799	6' 10-1/4"	1,423
HF-D18 15x8	11,563	0.410	12,951	0.459	28,896	0.370	12,278	5,243	6' 9-1/4"	1,890
HF-D18 <b>H</b> 15x8	23,208	0.410	25,993	0.459	38,640	0.370	20,283	10,500	6' 5-3/4"	2,437
HF-D9 16x8	5,596	0.410	7,834	0.574	31,640	0.334	9,209	2,498	6' 10-1/4"	1,462
HF-D18 16x8	10,732	0.410	12,020	0.459	27,048	0.390	11,773	4,594	6' 9-1/4"	1,938
HF-D18 <b>H</b> 16x8	21,612	0.410	24,205	0.459	31,416	0.392	18,273	9,241	6' 5-3/4"	2,502
HF-D18 <b>H</b> 17x8	20,200	0.410	22,624	0.459	33,768	0.411	18,530	8,183	6' 5-3/4"	2,567
HF-D18 <b>H</b> 18x8	18,952	0.410	21,226	0.459	35,280	0.431	18,706	7,299	6' 5-3/4"	2,633
HF-D18 <b>H</b> 19x8	17,835	0.410	19,975	0.459	36,960	0.451	19,025	6,542	6' 5-3/4"	2,698
HF-D18 <b>H</b> 20x8	16,840	0.410	18,861	0.459	31,920	0.472	17,570	5,900	6' 5-3/4"	2,763
HF-D18 <b>H</b> 21x8	15,938	0.410	17,851	0.459	31,920	0.492	17,504	5,337	6' 5-3/4"	2,828
HF-D18 <b>H</b> 22x8	15,125	0.410	16,940	0.459	31,248	0.513	17,288	4,855	6' 5-3/4"	2,893
HF-D18 <b>H</b> 23x8	14,389	0.410	16,116	0.459	30,408	0.534	17,046	4,440	6' 5-3/4"	2,958
							pinned base installation			

<sup>1.</sup> The values in this table are Allowable Stress Design (ASD) excluding a 1.33 stress increase and pertain to pinned base installation on 2500 psi concrete or nut & washers with high strength non-shrink grout

 $V_{R} = (V_{Dsn}/2) + (V_{Mult} \times Max P_{Dsn})$ 

 $\begin{array}{ll} \mbox{Where:} & \mbox{$V_{Dsn}$ = Design Shear Load (lbs)} \\ \mbox{$P_{Dsn}$ = Design Aggregate Point Load at the beam mid-span (lbs)} \end{array}$ 

6. The Uplift on a column is calculated with the following equation:

 $\mbox{Uplift} = \mbox{[(V$_{Dsn}$ x MF Height) / W$_{CL}$] - (Min P$_{Dsn}$ / 2)} \qquad \mbox{Where: V$_{Dsn}$ = Design Shear Load (lbs)}$ 

MF Height = Dimension from top of concrete to top of the wall framing (ft)

W<sub>CL</sub> = Inside steel to steel dimension (Win) + the column depth (ft)

 $P_{Dsn} = Design Aggregate Point Load at the beam mid-span (lbs)$ 

<sup>2.</sup> Drift at the allowable shear load is within the code allowed limits for R=6.5,  $C_d$ =4.0,  $\Delta_a$  = 0.025h

<sup>3.</sup> Pu is the allowable aggregate point load applied at the beam mid-span in combination with the vertical load produced by overturning at the allowable shear load.

 $<sup>4. \ \</sup> The \ table \ value \ for \ Resultant \ Shear \ (V_R) \ assumes \ the \ Allowable \ Shear \ (V) \ in \ combination \ with \ the \ Maximum \ Additional \ Vertical \ Load \ (Pu).$ 

<sup>5.</sup> The design Resultant Shear (V<sub>R</sub>) from combined horizontal and compression forces on a column are solved with the following equation:

<sup>7.</sup> h1 is the dimension from top of concrete to bottom of the lowest point in the header assembly assuming installation on nuts and washers with the Base Plate plate elevated 1-1/2" above finish concrete

### HARDY A FRAMES

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				Table 1.9	- 9' 1" Nomi	nal Wall Heig	hts	s Net Wall Ht = 9' 0-3/4"				
	Seism	nic	w	ind	Maximum Add'l	Shear Multiplier <sup>4</sup>	Column Base	Reactions	h1 <sup>7</sup>			
Model	Allowable <sup>1</sup> Shear V @ R= 6.5 (lbs)	Drift <sup>2</sup> @ Allow V (in)	Allowable Shear V (lbs)	Drift <sup>2</sup> @ Allow V (in)		@ Compression Column V <sub>Mult</sub>	Resultant Shear <sup>5</sup> VR @ Allow V & Max Pu (lbs)	Uplift <sup>6</sup> @ Allow V when Pu = 0 (lbs)	Opening Height (ft-in)	Approx Weight (lbs)		
HF-D9 6x9	6,232	0.463	8,725	0.648	50,680	0.129	7,081	7,420	8' 0-1/4"	1,064		
HF-D9 7x9	5,549	0.463	7,769	0.648	44,520	0.147	6,739	5,813	8' 0-1/4"	1,096		
HF-D9 8x9	4,980	0.463	6,972	0.648	39,760	0.165	6,471	4,661	8' 0-1/4"	1,128		
HF-D9 <b>H</b> 8x9	6,405	0.463	8,967	0.648	48,720	0.157	7,848	5,993	7' 11-1/4"	1,179		
HF-D12 8x9	5,357	0.463	7,500	0.648	40,040	0.175	6,929	4,868	8' 0-1/4"	938		
HF-D18 8x9	11,235	0.463	12,583	0.519	23,520	0.207	8,570	9,750	7' 11-3/4"	1,490		
HF-D9 9x9	5,878	0.463	8,229	0.648	44,240	0.175	7,630	4,969	7' 11-1/4"	1,214		
HF-D12 9x9	4,823	0.463	6,752	0.648	35,840	0.193	6,611	3,971	8' 0-1/4"	970		
HF-D18 9x9	17,144	0.463	19,201	0.519	33,600	0.217	13,006	13,533	7' 9-1/4"	1,680		
HF-D9 10x9	5,421	0.463	7,589	0.648	40,320	0.193	7,432	4,179	7' 11-1/4"	1,250		
HF-D12 10x9	4,375	0.463	6,125	0.648	32,480	0.211	6,350	3,295	8' 0-1/4"	1,002		
HF-D18 10x9	15,410	0.463	17,259	0.519	42,840	0.235	13,816	11,156	7' 9-1/4"	1,728		
HF-D9 11x9	5,022	0.463	7,031	0.648	36,960	0.210	7,231	3,556	7' 11-1/4"	1,285		
HF-D12 11x9	3,996	0.463	5,594	0.648	29,680	0.229	6,132	2,772	8' 0-1/4"	1,035		
HF-D18 11x9	13,951	0.463	15,625	0.519	39,144	0.253	12,990	9,328	7' 9-1/4"	1,776		
HF-D9 12x9	4,673	0.463	6,542	0.648	34,160	0.228	7,067	3,060	7' 11-1/4"	1,321		
HF-D12 12x9	3,672	0.463	5,141	0.648	27,440	0.247	5,945	2,357	8' 0-1/4"	1,067		
HF-D18 12x9	12,715	0.463	14,241	0.519	29,400	0.272	11,204	7,895	7' 9-1/4"	1,824		
HF-D9 13x9	4,365	0.463	6,111	0.648	31,640	0.246	6,903	2,658	7' 11-1/4"	1,357		
HF-D12 13x9	3,394	0.463	4,752	0.648	25,480	0.265	5,801	2,029	8' 0-1/4"	1,099		
HF-D18 13x9	11,655	0.463	13,054	0.519	32,760	0.289	11,575	6,757	7' 9-1/4"	1,871		
HF-D9 14x9	4,094	0.463	5,732	0.648	29,400	0.264	6,759	2,330	7' 11-1/4"	1,392		
HF-D12 14x9	3,153	0.463	4,414	0.648	23,520	0.283	5,610	1,768	8' 0-1/4"	1,131		
HF-D18 14x9	10,738	0.463	12,027	0.519	30,912	0.307	11,137	5,839	7' 9-1/4"	1,919		
HF-D18 <b>H</b> 14x9	21,233	0.463	23,781	0.519	42,504	0.306	18,500	11,525	7' 5-3/4"	2,471		
HF-D9 15x9	4,953	0.463	6,934	0.648	33,320	0.273	7,988	2,652	7' 10-1/4"	1,500		
HF-D18 15x9	9,950	0.463	11,144	0.519	28,728	0.325	10,646	5,089	7' 9-1/4"	1,967		
HF-D18 <b>H</b> 15x9	19,752	0.463	22,122	0.519	45,864	0.323	18,873	10,098	7' 5-3/4"	2,536		
HF-D9 16x9	4,701	0.463	6,581	0.648	31,080	0.291	7,837	2,371	7' 10-1/4"	1,539		
HF-D18 16x9	9,258	0.463	10,369	0.519	26,880	0.343	10,232	4,473	7' 9-1/4"	2,015		
HF-D18 <b>H</b> 16x9	18,442	0.463	20,655	0.519	35,280	0.342	16,548	8,906	7' 5-3/4"	2,601		
HF-D18 <b>H</b> 17x9	17,284	0.463	19,358	0.519	37,464	0.360	16,824	7,908	7' 5-3/4"	2,667		
HF-D18 <b>H</b> 18x9	16,249	0.463	18,199	0.519	38,136	0.378	16,866	7,065	7' 5-3/4"	2,732		
HF-D18 <b>H</b> 19x9	15,325	0.463	17,164	0.519	37,464	0.395	16,656	6,348	7' 5-3/4"	2,797		
HF-D18 <b>H</b> 20x9	14,490	0.463	16,229	0.519	33,768	0.414	15,731	5,732	7' 5-3/4"	2,862		
HF-D18 <b>H</b> 21x9	13,738	0.463	15,387	0.519	33,600	0.432	15,680	5,196	7' 5-3/4"	2,927		
HF-D18 <b>H</b> 22x9	13,058	0.463	14,625	0.519	32,088	0.451	15,308	4,734	7' 5-3/4"	2,993		
HF-D18 <b>H</b> 23x9	12,436	0.463	13,928	0.519	30,408	0.469	14,874	4,333	7' 5-3/4"	3,058		

<sup>1.</sup> The values in this table are Allowable Stress Design (ASD) excluding a 1.33 stress increase and pertain to pinned base installation on 2500 psi concrete or nut & washers with high strength non-shrink grout

$$V_R = (V_{Dsn}/2) + (V_{Mult} \times Max P_{Dsn})$$

Where:  $V_{Dsn} = Design Shear Load (lbs)$ 

 $P_{Dsn}^{SSS}$  = Design Aggregate Point Load at the beam mid-span (lbs)

6. The Uplift on a column is calculated with the following equation:

Uplift =  $[(V_{Dsn} \times MF \text{ Height}) / W_{CL}] - (Min P_{Dsn} / 2)$ 

**Where:**  $V_{Dsn} = Design Shear Load (lbs)$ 

MF Height = Dimension from top of concrete to top of the wall framing (ft)

 $W_{CL} =$ Inside steel to steel dimension (Win) + the column depth (ft)

P<sub>Dsn</sub> = Design Aggregate Point Load at the beam mid-span (lbs)

<sup>2.</sup> Drift at the allowable shear load is within the code allowed limits for R=6.5,  $C_d$ =4.0,  $\Delta_a$  = 0.025h

<sup>3.</sup> Pu is the allowable aggregate point load applied at the beam mid-span in combination with the vertical load produced by overturning at the allowable shear load.

<sup>4.</sup> The table value for Resultatnt Shear (V<sub>R</sub>) assumes the Allowable Shear (V) in combination with the Maximum Additional Vertical Load (Pu).

<sup>5.</sup> The design Resultant Shear  $(V_R)$  from combined horizontal and compression forces on a column are solved with the following equation:

<sup>7.</sup> h1 is the dimension from top of concrete to bottom of the lowest point in the header assembly assuming installation on nuts and washers with the Base Plate plate elevated 1-1/2" above finish concrete

### HARDY A FRAMES.

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				Table '	1.10 - 10' 1" N	Iominal Wall H	eights	Net Wal	I Ht = 10' 0	-3/4"
	Seisn	nic	Win	nd	Maximum Add'l	Choor Multiplie	Column Base I	Reactions	h1 <sup>7</sup>	
Model	Allowable <sup>1</sup> Shear V @ R= 6.5 (lbs)	Drift <sup>2</sup> @ Allow V (in)	Allowable Shear V (lbs)	Drift <sup>2</sup> @ Allow V (in)	Vertical <sup>3</sup> Load Pu (lbs)	Shear Multiplier <sup>4</sup> @ Compression Column V <sub>Mult</sub>	Resultant Shear <sup>5</sup> VR @ Allow V & Max Pu (lbs)	Uplift <sup>6</sup> @ Allow V when Pu = 0 (lbs)	Opening Height (ft-in)	Approx Weight (lbs)
HF-D9 6x10	5,220	0.516	7,308	0.722	49,560	0.113	6,002	6,931	9' 0-1/4"	1,141
HF-D9 7x10	4,683	0.516	6,556	0.722	43,680	0.129	5,764	5,471	9' 0-1/4"	1,173
HF-D9 8x10	4,230	0.516	5,922	0.722	39,200	0.145	5,576	4,413	9' 0-1/4"	1,205
HF-D9 <b>H</b> 8x10	5,360	0.516	7,504	0.722	47,600	0.137	6,645	5,592	8' 11-1/4"	1,256
HF-D12 8x10	4,558	0.516	6,381	0.722	39,200	0.155	5,963	4,621	9' 0-1/4"	987
HF-D18 8x10	9,743	0.516	10,912	0.578	27,720	0.184	7,971	9,422	8' 11-3/4"	1,567
HF-D9 9x10	4,948	0.516	6,927	0.722	43,400	0.153	6,508	4,661	8' 11-1/4"	1,291
HF-D12 9x10	4,126	0.516	5,776	0.722	35,560	0.171	5,753	3,790	9' 0-1/4"	1,019
HF-D18 9x10	14,625	0.516	16,380	0.578	45,360	0.192	12,599	12,864	8' 9-1/4"	1,757
HF-D9 10x10	4,584	0.516	6,418	0.722	39,480	0.170	6,357	3,938	8' 11-1/4"	1,327
HF-D12 10x10	3,759	0.516	5,263	0.722	32,200	0.187	5,532	3,154	9' 0-1/4"	1,051
HF-D18 10x10	13,225	0.516	14,812	0.578	42,168	0.208	11,944	10,667	8' 9-1/4"	1,805
HF-D9 11x10	4,266	0.516	5,972	0.722	36,120	0.185	6,196	3,368	8' 11-1/4"	1,362
HF-D12 11x10	3,447	0.516	4,826	0.722	29,400	0.203	5,348	2,665	9' 0-1/4"	1,115
HF-D18 11x10	12,023	0.516	13,466	0.578	38,640	0.225	11,282	8,960	8' 9-1/4"	1,853
HF-D9 12x10	3,984	0.516	5,578	0.722	33,600	0.202	6,106	2,913	8' 11-1/4"	1,398
HF-D12 12x10	3,178	0.516	4,449	0.722	27,160	0.219	5,204	2,277	9' 0-1/4"	1,115
HF-D18 12x10	10,994	0.516	12,313	0.578	33,600	0.241	10,412	7,607	8' 9-1/4"	1,901
HF-D9 13x10	3,735	0.516	5,229	0.722	31,080	0.217	5,968	2,538	8' 11-1/4"	1,434
HF-D12 13x10	2,945	0.516	4,123	0.722	25,200	0.235	5,073	1,962	9' 0-1/4"	1,148
HF-D18 13x10	10,109	0.516	11,322	0.578	33,096	0.257	10,223	6,536	8' 9-1/4"	1,949
HF-D9 14x10	3,512	0.516	4,917	0.722	29,120	0.233	5,881	2,230	8' 11-1/4"	1,469
HF-D12 14x10	2,742	0.516	3,839	0.722	23,240	0.251	4,913	1,714	9' 0-1/4"	1,180
HF-D18 14x10	9,344	0.516	10,465	0.578	30,744	0.273	9,768	5,665	8' 9-1/4"	1,997
HF-D18 <b>H</b> 14x10	18,255	0.516	20,446	0.578	48,720	0.270	17,127	11,049	8' 5-3/4"	2,570
HF-D9 15x10	4,212	0.516	5,897	0.722	32,760	0.240	6,879	2,511	8' 10-1/4"	1,577
HF-D18 15x10	8,673	0.516	9,714	0.578	28,560	0.289	9,355	4,949	8' 9-1/4"	2,045
HF-D18 <b>H</b> 15x10	17,034	0.516	19,078	0.578	47,376	0.287	16,757	9,703	8' 5-3/4"	2,636
HF-D9 16x10	4,008	0.516	5,611	0.722	30,800	0.256	6,789	2,250	8' 10-1/4"	1,616
HF-D18 16x10	8,086	0.516	9,056	0.578	26,880	0.306	9,031	4,359	8' 9-1/4"	2,093
HF-D18 <b>H</b> 16x10	15,950	0.516	17,864	0.578	39,816	0.303	15,302	8,585	8' 5-3/4"	2,701
HF-D18 <b>H</b> 17x10	14,982	0.516	16,780	0.578	41,160	0.319	15,462	7,641	8' 5-3/4"	2,766
HF-D18 <b>H</b> 18x10	14,116	0.516	15,810	0.578	39,480	0.335	15,093	6,844	8' 5-3/4"	2,831
HF-D18 <b>H</b> 19x10	13,337	0.516	14,937	0.578	37,296	0.352	14,626	6,161	8' 5-3/4"	2,896
HF-D18 <b>H</b> 20x10	12,632	0.516	14,148	0.578	35,448	0.368	14,236	5,571	8' 5-3/4"	2,962
HF-D18 <b>H</b> 21x10	11,995	0.516	13,434	0.578	33,600	0.384	13,837	5,056	8' 5-3/4"	3,027
HF-D18 <b>H</b> 22x10	11,414	0.516	12,784	0.578	31,920	0.401	13,477	4,614	8' 5-3/4"	3,092
HF-D18 <b>H</b> 23x10	10,886	0.516	12,192	0.578	30,408	0.418	13,155	4,225	8' 5-3/4"	3,157
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<sup>1.</sup> The values in this table are Allowable Stress Design (ASD) excluding a 1.33 stress increase and pertain to pinned base installation on 2500 psi concrete or nut & washers with high strength non-shrink grout

 $V_R = (V_{Dsn}/2) + (V_{Mult} \times Max \stackrel{..}{P}_{Dsn})$ 

Where:  $V_{Dsn} = Design Shear Load (lbs)$ 

P<sub>Dsn</sub> = Design Aggregate Point Load at the beam mid-span (lbs)

6. The Uplift on a column is calculated with the following equation:

Uplift =  $[(V_{Dsn} \times MF \text{ Height}) / W_{CL}] - (Min P_{Dsn} / 2)$ 

Where:  $V_{Dsn} = Design Shear Load (lbs)$ 

MF Height = Dimension from top of concrete to top of the wall framing (ft)

 $W_{CL} = Inside$  steel to steel dimension (Win) + the column depth (ft)

 $P_{Dsn}^{3}$  = Design Aggregate Point Load at the beam mid-span (lbs)

7. h1 is the dimension from top of concrete to bottom of the lowest point in the header assembly assuming installation on nuts and washers with the Base Plate plate elevated 1-1/2" above finish concrete

<sup>2.</sup> Drift at the allowable shear load is within the code allowed limits for R=6.5,  $C_d$ =4.0,  $\Delta_a$  = 0.025h

<sup>3.</sup> Pu is the allowable aggregate point load applied at the beam mid-span in combination with the vertical load produced by overturning at the allowable shear load.

<sup>4.</sup> The table value for Resultatnt Shear (V<sub>R</sub>) assumes the Allowable Shear (V) in combination with the Maximum Additional Vertical Load (Pu).

<sup>5.</sup> The design Resultant Shear (V<sub>R</sub>) from combined horizontal and compression forces on a column are solved with the following equation:

	Table 1.11 - 11' 1" Nominal Wall Heights Net Wall Ht = 11' 0-3/4"									
	Seism	ic	w	ind	Maximum Add'l	Shear Multiplier <sup>4</sup>	Column Base	Reactions	h1 <sup>7</sup> Opening	Approx
Model	Allowable <sup>1</sup> Shear V @ R= 6.5 (lbs)	Drift2 @ Allow V (in)		Drift <sup>2</sup> @ Allow V (in)	Vertical <sup>3</sup> Load Pu (lbs)	@ Compression Column V <sub>Mult</sub>	Resultant Shear <sup>5</sup> VR @ Allow V & Max Pu (lbs)		Height (ft-in)	Weight (lbs)
HF-D9 6x11	4,436	0.569	6,210	0.797	48,440	0.100	5,157	6,496	10' 0-1/4"	1,218
HF-D9 7x11	4,007	0.569	5,610	0.797	42,840	0.115	4,982	5,163	10' 0-1/4"	1,250
HF-D9 8x11	3,640	0.569	5,096	0.797	38,360	0.130	4,841	4,185	10' 0-1/4"	1,282
HF-D9 <b>H</b> 8x11	4,552	0.569	6,373	0.797	46,480	0.121	5,697	5,237	9' 11-1/4"	1,333
HF-D12 8x11	3,929	0.569	5,501	0.797	38,640	0.138	5,209	4,393	10' 0-1/4"	1,035
HF-D18 8x11	8,549	0.569	9,575	0.637	32,424	0.165	7,531	9,121	9' 11-3/4"	1,644
HF-D9 9x11	4,222	0.569	5,911	0.797	42,280	0.136	5,606	4,386	9' 11-1/4"	1,368
HF-D12 9x11	3,573	0.569	5,002	0.797	35,000	0.153	5,038	3,616	10' 0-1/4"	1,068
HF-D18 9x11	12,635	0.569	14,151	0.637	45,864	0.172	11,103	12,261	9' 9-1/4"	1,834
HF-D9 10x11	3,930	0.569	5,502	0.797	38,640	0.151	5,499	3,723	9' 11-1/4"	1,404
HF-D12 10x11	3,269	0.569	4,577	0.797	31,920	0.168	4,884	3,027	10' 0-1/4"	1,100
HF-D18 10x11	11,474	0.569	12,851	0.637	41,832	0.187	10,479	10,212	9' 9-1/4"	1,882
HF-D9 11x11	3,671	0.569	5,139	0.797	35,560	0.165	5,395	3,194	9' 11-1/4"	1,439
HF-D12 11x11	3,008	0.569	4,211	0.797	29,120	0.182	4,727	2,565	10' 0-1/4"	1,132
HF-D18 11x11	10,478	0.569	11,735	0.637	38,304	0.201	9,923	8,612	9' 9-1/4"	1,930
HF-D9 12x11	3,440	0.569	4,816	0.797	33,040	0.180	5,321	2,772	9' 11-1/4"	1,475
HF-D12 12x11	2,782	0.569	3,895	0.797	26,880	0.196	4,593	2,196	10' 0-1/4"	1,164
HF-D18 12x11	9,616	0.569	10,770	0.637	35,280	0.216	9,441	7,339	9' 9-1/4"	1,978
HF-D9 13x11	3,234	0.569	4,528	0.797	30,800	0.194	5,251	2,424	9' 11-1/4"	1,510
HF-D12 13x11	2,585	0.569	3,619	0.797	24,920	0.211	4,482	1,902	10' 0-1/4"	1,196
HF-D18 13x11	8,867	0.569	9,931	0.637	32,760	0.231	9,030	6,321	9' 9-1/4"	2,026
HF-D9 14x11	3,050	0.569	4,270	0.797	28,840	0.209	5,177	2,136	9' 11-1/4"	1,546
HF-D12 14x11	2,412	0.569	3,377	0.797	23,240	0.226	4,395	1,661	10' 0-1/4"	1,228
HF-D18 14x11	8,215	0.569	9,201	0.637	30,408	0.246	8,642	5,491	9' 9-1/4"	2,074
HF-D18 <b>H</b> 14x11	15,878	0.569	17,783	0.637	50,232	0.242	15,323	10,594	9' 5-3/4"	2,670
HF-D9 15x11	3,629	0.569	5,081	0.797	32,480	0.214	6,025	2,384	9' 10-1/4"	1,654
HF-D18 15x11	7,644	0.569	8,561	0.637	28,392	0.260	8,306	4,808	9' 9-1/4"	2,122
HF-D18 <b>H</b> 15x11	14,856	0.569	16,639	0.637	47,040	0.257	14,766	9,335	9' 5-3/4"	2,735
HF-D9 16x11	3,461	0.569	4,845	0.797	30,520	0.228	5,954	2,143	9' 10-1/4"	1,693
HF-D18 16x11	7,140	0.569	7,997	0.637	26,712	0.275	8,034	4,246	9' 9-1/4"	2,170
HF-D18 <b>H</b> 16x11	13,947	0.569	15,621	0.637	44,184	0.272	14,262	8,277	9' 5-3/4"	2,800
HF-D18 <b>H</b> 17x11	13,128	0.569	14,703	0.637	41,664	0.287	13,812	7,386	9' 5-3/4"	2,865
HF-D18 <b>H</b> 18x11	12,394	0.569	13,881	0.637	39,312	0.301	13,384	6,623	9' 5-3/4"	2,930
HF-D18 <b>H</b> 19x11	11,731	0.569	13,139	0.637	37,128	0.316	12,989	5,973	9' 5-3/4"	2,996
HF-D18 <b>H</b> 20x11	11,131	0.569	12,467	0.637	35,280	0.331	12,651	5,411	9' 5-3/4"	3,061
HF-D18 <b>H</b> 21x11	10,584	0.569	11,854	0.637	33,600	0.346	12,341	4,922	9' 5-3/4"	3,126
HF-D18 <b>H</b> 22x11	10,086	0.569	11,296	0.637	31,920	0.361	12,033	4,500	9' 5-3/4"	3,191
HF-D18 <b>H</b> 23x11	9,630	0.569	10,786	0.637	30,408	0.376	11,757	4,125	9' 5-3/4"	3,256
1 The values in th	nic table are Allo	wahla Strace	Decian (ASD)	eveluding a 1	33 etrace incres	ca and partain to ni	nned hase installation (	n 2500 nei conero	to or nut & was	chore

<sup>1.</sup> The values in this table are Allowable Stress Design (ASD) excluding a 1.33 stress increase and pertain to pinned base installation on 2500 psi concrete or nut & washers with high strength non-shrink grout

Where:

 $V_R = (V_{Dsn}/2) + (V_{Mult} \times Max P_{Dsn})$ 

 $V_{Dsn}$  = Design Shear Load (lbs)

P<sub>Dsn</sub> = Design Aggregate Point Load at the beam mid-span (lbs)

6. The Uplift on a column is calculated with the following equation:

Uplift =  $[(V_{Dsn} \times MF \text{ Height}) / W_{CL}] - (Min P_{Dsn} / 2)$ 

 $\label{eq:Where: VDsn} \begin{tabular}{ll} Where: VDsn = Design Shear Load (lbs) \\ MF Height = Dimension from top of concrete to top of the wall framing (ft) \\ \end{tabular}$ 

<sup>2.</sup> Drift at the allowable shear load is within the code allowed limits for R=6.5,  $C_d$ =4.0,  $\Delta_a$  = 0.025h

 <sup>3.</sup> Pu is the allowable aggregate point load applied at the beam mid-span in combination with the vertical load produced by overturning at the allowable shear load.
 4. The table value for Resultatnt Shear (V<sub>R</sub>) assumes the Allowable Shear (V) in combination with the Maximum Additional Vertical Load (Pu).

<sup>5.</sup> The design Resultant Shear (V<sub>R</sub>) from combined horizontal and compression forces on a column are solved with the following equation:

To be seen unitiension (Win) + the column depth (ft)

Posn = Design Aggregate Point Load at the beam mid-span (lbs)

7. h1 is the dimension from top of concrete to bottom of the lowest point in the header assembly assuming installation on nuts and washers with the Base Plate plate elevated 1-1/2" above finish concrete

10 Hardy Frame

### HARDY A FRAMES.

A MiTek Company

				<b>Table 1.12</b>	? - 12' 1" Nor	minal Wall Heig	jhts	Net Wall	Ht = 12' 0	-3/4"
	Seism	nic	Wi	ind	Maximum Add'l	Shear Multiplier <sup>4</sup>			Approx	
Model	Allowable <sup>1</sup> Shear V @ R= 6.5 (lbs)	Drift <sup>2</sup> @ Allow V (in)	Allowable Shear V (lbs)	Drift <sup>2</sup> @ Allow V (in)	Vertical <sup>3</sup> Load Pu (lbs)	@ Compression Column V <sub>Mult</sub>	Resultant Shear <sup>5</sup> VR @ Allow V & Max Pu (lbs)		Height (ft-in)	Weight (lbs)
HF-D9 6x12	3,816	0.622	5,342	0.871	47,320	0.089	4,478	6,107	11' 0-1/4"	1,295
HF-D9 7x12	3,468	0.622	4,855	0.871	42,000	0.103	4,364	4,888	11' 0-1/4"	1,327
HF-D9 8x12	3,165	0.622	4,431	0.871	37,800	0.117	4,257	3,984	11' 0-1/4"	1,359
HF-D9 <b>H</b> 8x12	3,912	0.622	5,477	0.871	45,640	0.108	4,955	4,922	10' 11-1/4"	1,410
HF-D12 8x12	3,423	0.622	3,834	0.697	22,848	0.125	3,442	4,185	11' 0-1/4"	1,084
HF-D18 8x12	7,568	0.622	8,476	0.697	35,112	0.150	6,975	8,826	10' 11-3/4"	1,722
HF-D9 9x12	3,646	0.622	5,104	0.871	41,720	0.122	4,908	4,145	10' 11-1/4"	1,445
HF-D12 9x12	3,127	0.622	3,502	0.697	20,664	0.138	3,295	3,462	11' 0-1/4"	1,116
HF-D18 9x12	11,022	0.622	12,345	0.697	45,192	0.155	9,766	11,692	10' 9-1/4"	1,911
HF-D9 10x12	3,406	0.622	4,768	0.871	38,080	0.135	4,828	3,529	10' 11-1/4"	1,481
HF-D12 10x12	2,871	0.622	3,216	0.697	18,816	0.151	3,162	2,906	11' 0-1/4"	1,148
HF-D18 10x12	10,058	0.622	11,265	0.697	41,160	0.169	9,248	9,783	10' 9-1/4"	1,959
HF-D9 11x12	3,194	0.622	4,472	0.871	35,280	0.148	4,770	3,040	10' 11-1/4"	1,516
HF-D12 11x12	2,650	0.622	2,968	0.697	17,304	0.165	3,054	2,471	11' 0-1/4"	1,181
HF-D18 11x12	9,219	0.622	10,325	0.697	37,800	0.182	8,790	8,283	10' 9-1/4"	2,007
HF-D9 12x12	3,002	0.622	4,203	0.871	32,480	0.161	4,680	2,645	10' 11-1/4"	1,552
HF-D12 12x12	2,458	0.622	2,753	0.697	15,960	0.178	2,954	2,123	11' 0-1/4"	1,213
HF-D18 12x12	8,490	0.622	9,509	0.697	34,944	0.196	8,404	7,085	10' 9-1/4"	2,055
HF-D9 13x12	2,830	0.622	3,962	0.871	30,240	0.175	4,620	2,317	10' 11-1/4"	1,587
HF-D12 13x12	2,290	0.622	2,565	0.697	14,784	0.191	2,861	1,842	11' 0-1/4"	1,245
HF-D18 13x12	7,850	0.622	8,792	0.697	32,424	0.209	8,047	6,114	10' 9-1/4"	2,103
HF-D9 14x12	2,676	0.622	3,746	0.871	28,280	0.188	4,569	2,049	10' 11-1/4"	1,623
HF-D12 14x12	2,142	0.622	2,399	0.697	13,776	0.204	2,781	1,614	11' 0-1/4"	1,277
HF-D18 14x12	7,290	0.622	8,165	0.697	30,240	0.223	7,736	5,330	10' 9-1/4"	2,151
HF-D18 <b>H</b> 14x12	13,942	0.622	15,615	0.697	49,728	0.219	13,578	10,172	10' 5-3/4"	2,769
HF-D9 15x12	3,159	0.622	4,423	0.871	31,920	0.191	5,280	2,270	10' 10-1/4"	1,731
HF-D18 15x12	6,798	0.622	7,614	0.697	28,224	0.236	7,446	4,674	10' 9-1/4"	2,199
HF-D18 <b>H</b> 15x12	13,081	0.622	14,651	0.697	46,536	0.232	13,107	8,987	10' 5-3/4"	2,834
HF-D9 16x12	3,021	0.622	4,229	0.871	29,960	0.205	5,243	2,049	10' 10-1/4"	1,770
HF-D18 16x12	6,363	0.622	7,127	0.697	26,544	0.250	7,214	4,138	10' 9-1/4"	2,247
HF-D18 <b>H</b> 16x12	12,307	0.622	13,784	0.697	43,680	0.246	12,671	7,989	10' 5-3/4"	2,899
HF-D18 <b>H</b> 17x12	11,610	0.622	13,003	0.697	41,328	0.259	12,311	7,138	10' 5-3/4"	2,965
HF-D18 <b>H</b> 18x12	10,983	0.622	12,301	0.697	38,976	0.273	11,954	6,422	10' 5-3/4"	3,030
HF-D18 <b>H</b> 19x12	10,412	0.622	11,661	0.697	36,960	0.287	11,637	5,799	10' 5-3/4"	3,095
HF-D18 <b>H</b> 20x12	9,895	0.622	11,082	0.697	35,112	0.300	11,345	5,257	10' 5-3/4"	3,160
HF-D18 <b>H</b> 21x12	9,423	0.622	10,554	0.697	33,432	0.314	11,087	4,795	10' 5-3/4"	3,225
HF-D18 <b>H</b> 22x12	8,991	0.622	10,070	0.697	31,752	0.327	10,806	4,386	10' 5-3/4"	3,290
HF-D18 <b>H</b> 23x12	8,594	0.622	9,625	0.697	30,240	0.342	10,570	4,025	10' 5-3/4"	3,356

<sup>1.</sup> The values in this table are Allowable Stress Design (ASD) excluding a 1.33 stress increase and pertain to pinned base installation on 2500 psi concrete or nut & washers with high strength non-shrink grout

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 $P_{Dsn}^{\rm DSS} =$  Design Aggregate Point Load at the beam mid-span (lbs) uation:

6. The Uplift on a column is calculated with the following equation:

MF Height = Dimension from top of concrete to top of the wall framing (ft)

W<sub>CL</sub> = Inside steel to steel dimension (Win) + the column depth (ft)

 $P_{Dsn}$  = Design Aggregate Point Load at the beam mid-span (lbs)

<sup>2.</sup> Drift at the allowable shear load is within the code allowed limits for R=6.5,  $C_d$ =4.0,  $\Delta_a$  = 0.025h

<sup>3.</sup> Pu is the allowable aggregate point load applied at the beam mid-span in combination with the vertical load produced by overturning at the allowable shear load.

 $<sup>4. \ \, \</sup>text{The table value for Resultatnt Shear ($V_R$) assumes the Allowable Shear ($V$) in combination with the Maximum Additional Vertical Load ($P_U$)}.$ 

<sup>5.</sup> The design Resultant Shear  $(V_R)$  from combined horizontal and compression forces on a column are solved with the following equation:  $V_R = (V_{Dsn}/2) + (V_{Mult} \times Max P_{Dsn})$  Where:  $V_{Dsn} = Design Shear Load (lbs)$ 

<sup>7.</sup> h1 is the dimension from top of concrete to bottom of the lowest point in the header assembly assuming installation on nuts and washers with the Base Plate plate elevated 1-1/2" above finish concrete

Mode   Mode   Microsoft   Nationable   National Points   Nationa	Table 1.13 - 13' 1" Nominal Wall Heights Net Wall Ht = 1									lt = 13' 0-3	3/4"
HF-D9 6x13   3,317   0,540   4,644   0,756   41,440   0,093   3,860   4,634   12' 0,14"   1,43   1,45   1		Seism	ic	Wi	ind			Column Base I	Reactions		Approx
HF-D9 7x13   3,031   0,540   4,243   0,756   41,440   0.093   3,860   4,634   12 0-1/4"   1,40   HF-D9 8x13   2,780   0,540   3,892   0,756   37,240   0,105   3,773   3,797   3,991   12 0-1/4"   1,43   1,48   HF-D12 8x13   3,099   0,540   3,370   0,605   18,816   0,113   2,797   3,991   12 0-1/4"   1,43   1,48   HF-D12 8x13   3,099   0,540   3,370   0,605   18,816   0,113   2,797   3,991   12 0-1/4"   1,13   HF-D18 8x13   6,754   0,540   7,564   0,605   34,776   0,137   6,274   8,551   11" 11-3/4"   1,79   HF-D12 9x13   3,179   0,540   3,091   0,606   18,884   0,110   4,314   3,924   11" 11-1/4"   1,85   1,17	Model	Shear V @					@ Compression Column V <sub>Mult</sub>			Height	Weight (lbs)
HF-D9 8x13	HF-D9 6x13	3,317	0.540	4,644	0.756	46,480	0.081	3,931	5,766	12' 0-1/4"	1,372
HF-D9H 8x13   3,399   0.540   4,759   0.756   44,800   0.098   4,351   4,641   11' 11-1/4' 1,48   HF-D12 8x13   3,009   0.540   3,370   0.605   18,816   0.113   2,797   3,991   12' 0-1/4'   1,13   4,641   11' 11-1/4' 1,79   1,48   1,49	HF-D9 7x13	3,031	0.540	4,243	0.756	41,440	0.093	3,860	4,634	12' 0-1/4"	1,404
HF-D12 8x13   3,009   0.540   3,370   0.605   18,816   0.113   2,797   3,991   12' 0-1/4"   1,13   1,14   1,14   1,15   1,14   1,14   1,15   1,14   1,15   1,14   1,15   1,14   1,15   1,14   1,15   1,14   1,14   1,15   1,14   1,14   1,15   1,14   1,14   1,15   1,14   1,14   1,15   1,14   1,14   1,15   1,14   1,14   1,15   1,14   1,14   1,14   1,14   1,15   1,14	HF-D9 8x13	2,780	0.540	3,892	0.756	37,240	0.105	3,773	3,797	12' 0-1/4"	1,436
HF-D18 8x13	HF-D9 <b>H</b> 8x13	3,399	0.540	4,759	0.756	44,800	0.098	4,351	4,641	11' 11-1/4"	1,487
HF-D9 9x13	HF-D12 8x13	3,009	0.540	3,370	0.605	18,816	0.113	2,797	3,991	12' 0-1/4"	1,133
HF-D12 9x13	HF-D18 8x13	6,754	0.540	7,564	0.605	34,776	0.137	6,274	8,551	11' 11-3/4"	1,799
HF-D18 9x13	HF-D9 9x13	3,179	0.540	4,451	0.756	40,880	0.110	4,314	3,924	11' 11-1/4"	1,522
HF-D9 10x13	HF-D12 9x13	2,760	0.540	3,091	0.605	18,984	0.125	2,819	3,315	12' 0-1/4"	1,165
HF-D12 10x13	HF-D18 9x13	9,705	0.540	10,870	0.605	44,520	0.141	8,659	11,170	11' 9-1/4"	1,989
HF-D18 10x13 8,893 0.540 9,960 0.605 40,656 0.154 8,239 9,388 11' 9-1/4" 2,03   HF-D9 11x13 2,804 0.540 3,926 0.756 34,720 0.134 4,236 2,900 11' 11-11/4" 1,59   HF-D12 11x13 2,355 0.540 2,638 0.605 17,136 0.150 2,733 2,384 12' 0-1/4" 1,22   HF-D18 11x13 8,182 0.540 9,164 0.605 37,464 0.166 7,872 7,975 11' 9-1/4" 2,08   HF-D9 12x13 2,643 0.540 3,700 0.756 32,200 0.147 4,186 2,525 11' 11-1/4" 1,62   HF-D12 12x13 2,190 0.540 2,453 0.605 15,792 0.162 2,652 2,049 12' 0-1/4" 1,22   HF-D18 12x13 7,555 0.540 8,462 0.605 34,608 0.179 7,539 6,844 11' 9-1/4" 2,13   HF-D9 13x13 2,498 0.540 3,497 0.756 29,960 0.159 4,132 2,223 11' 11-1/4" 1,66   HF-D12 13x13 7,004 0.540 7,844 0.605 32,088 0.192 7,235 5,926 11' 9-1/4" 1,29   HF-D18 13x13 7,004 0.540 7,844 0.605 32,088 0.192 7,235 5,926 11' 9-1/4" 1,29   HF-D18 14x13 1,916 0.540 3,315 0.756 28,000 0.171 4,085 1,969 11' 11-1/4" 1,70   HF-D18 14x13 1,916 0.540 2,146 0.605 13,776 0.187 2,518 1,567 12' 0-1/4" 1,32   HF-D18 14x13 1,2,346 0.540 13,828 0.605 49,056 0.199 12,103 9,777 11' 5-3/4" 2,28   HF-D18 15x13 2,776 0.540 13,828 0.605 49,056 0.199 12,103 9,777 11' 5-3/4" 2,86   HF-D18 15x13 5,713 0.540 6,823 0.605 28,056 0.216 6,725 4,547 11' 9-1/4" 2,38   HF-D18 15x13 5,713 0.540 6,823 0.605 28,680 0.186 4,673 1,955 11' 10-1/4" 1,84   HF-D18 16x13 5,713 0.540 6,399 0.605 26,376 0.229 6,519 4,031 11' 9-1/4" 2,39   HF-D18 16x13 10,949 0.540 11,593 0.605 40,992 0.237 11,069 6,911 11' 5-3/4" 2,99   HF-D18 16x13 10,351 0.540 11,593 0.605 40,992 0.237 11,069 6,911 11' 5-3/4" 2,99	HF-D9 10x13	2,982	0.540	4,175	0.756	37,520	0.122	4,273	3,355	11' 11-1/4"	1,558
HF-D9 11x13	HF-D12 10x13	2,545	0.540	2,850	0.605	18,648	0.137	2,826	2,792	12' 0-1/4"	1,197
HF-D12 11x13	HF-D18 10x13	8,893	0.540	9,960	0.605	40,656	0.154	8,239	9,388	11' 9-1/4"	2,037
HF-D18 11x13 8,182 0.540 9,164 0.605 37,464 0.166 7,872 7,975 11'9-1/4" 2,08 HF-D9 12x13 2,643 0.540 3,700 0.756 32,200 0.147 4,186 2,525 11' 11-1/4" 1,62 HF-D12 12x13 2,190 0.540 2,453 0.605 15,792 0.162 2,652 2,049 12' 0-1/4" 1,26 HF-D18 12x13 7,555 0.540 8,462 0.605 34,608 0.179 7,539 6,844 11' 9-1/4" 2,13 HF-D9 13x13 2,498 0.540 3,497 0.756 29,960 0.159 4,132 2,223 11' 11-1/4" 1,66 HF-D12 13x13 2,045 0.540 2,290 0.605 14,784 0.174 2,585 1,788 12' 0-1/4" 1,29 HF-D18 13x13 7,004 0.540 7,844 0.605 32,088 0.192 7,235 5,926 11' 9-1/4" 2,18 HF-D9 14x13 2,368 0.540 3,315 0.756 28,000 0.171 4,085 1,969 11' 11-1/4" 1,70 HF-D12 14x13 1,916 0.540 2,146 0.605 13,776 0.187 2,518 1,567 12' 0-1/4" 1,32 HF-D18 14x13 6,520 0.540 7,302 0.605 29,904 0.204 6,957 5,170 11' 9-1/4" 2,22 HF-D18 14x13 12,346 0.540 13,828 0.605 49,056 0.199 12,103 9,777 11' 5-3/4" 2,86 HF-D9 15x13 2,776 0.540 3,886 0.756 31,640 0.174 4,723 2,170 11' 10-1/4" 1,80 HF-D18 15x13 6,092 0.540 6,823 0.605 28,056 0.216 6,725 4,547 11' 9-1/4" 2,27 HF-D18 15x13 11,612 0.540 13,005 0.605 29,680 0.186 4,673 1,955 11' 10-1/4" 1,84 HF-D18 16x13 5,713 0.540 6,399 0.605 26,376 0.229 6,519 4,031 11' 9-1/4" 2,32 HF-D18 16x13 10,949 0.540 11,593 0.605 40,992 0.237 11,069 6,911 11' 5-3/4" 3,06	HF-D9 11x13	2,804	0.540	3,926	0.756	34,720	0.134	4,236	2,900	11' 11-1/4"	1,593
HF-D9 12x13	HF-D12 11x13	2,355	0.540	2,638	0.605	17,136	0.150	2,733	2,384	12' 0-1/4"	1,229
HF-D12 12x13	HF-D18 11x13	8,182	0.540	9,164	0.605	37,464	0.166	7,872	7,975	11' 9-1/4"	2,085
HF-D18 12x13         7,555         0.540         8,462         0.605         34,608         0.179         7,539         6,844         11' 9-1/4"         2,13           HF-D9 13x13         2,498         0.540         3,497         0.756         29,960         0.159         4,132         2,223         11' 11-1/4"         1,66           HF-D12 13x13         2,045         0.540         2,290         0.605         14,784         0.174         2,585         1,788         12' 0-1/4"         1,29           HF-D18 13x13         7,004         0.540         7,844         0.605         32,088         0.192         7,235         5,926         11' 9-1/4"         2,18           HF-D9 14x13         2,368         0.540         3,315         0.756         28,000         0.171         4,085         1,969         11' 11-1/4"         1,70           HF-D12 14x13         1,916         0.540         2,146         0.605         13,776         0.187         2,518         1,567         12' 0-1/4"         1,32           HF-D18 14x13         6,520         0.540         7,302         0.605         29,904         0.204         6,957         5,170         11' 9-1/4"         2,22           HF-D18 H 14x13         12,	HF-D9 12x13	2,643	0.540	3,700	0.756	32,200	0.147	4,186	2,525	11' 11-1/4"	1,629
HF-D9 13x13         2,498         0.540         3,497         0.756         29,960         0.159         4,132         2,223         11' 11-1/4"         1,66           HF-D12 13x13         2,045         0.540         2,290         0.605         14,784         0.174         2,585         1,788         12' 0-1/4"         1,29           HF-D18 13x13         7,004         0.540         7,844         0.605         32,088         0.192         7,235         5,926         11' 9-1/4"         2,18           HF-D9 14x13         2,368         0.540         3,315         0.756         28,000         0.171         4,085         1,969         11' 11-1/4"         1,70           HF-D12 14x13         1,916         0.540         2,146         0.605         13,776         0.187         2,518         1,567         12' 0-1/4"         1,32           HF-D18 14x13         6,520         0.540         7,302         0.605         29,904         0.204         6,957         5,170         11' 9-1/4"         2,22           HF-D18 1 14x13         12,346         0.540         13,828         0.605         49,056         0.199         12,103         9,777         11' 5-3/4"         2,86           HF-D18 1 5x13 <td< td=""><td>HF-D12 12x13</td><td>2,190</td><td>0.540</td><td>2,453</td><td>0.605</td><td>15,792</td><td>0.162</td><td>2,652</td><td>2,049</td><td>12' 0-1/4"</td><td>1,261</td></td<>	HF-D12 12x13	2,190	0.540	2,453	0.605	15,792	0.162	2,652	2,049	12' 0-1/4"	1,261
HF-D12 13x13         2,045         0.540         2,290         0.605         14,784         0.174         2,585         1,788         12' 0-1/4"         1,29           HF-D18 13x13         7,004         0.540         7,844         0.605         32,088         0.192         7,235         5,926         11' 9-1/4"         2,18           HF-D9 14x13         2,368         0.540         3,315         0.756         28,000         0.171         4,085         1,969         11' 11-1/4"         1,70           HF-D12 14x13         1,916         0.540         2,146         0.605         13,776         0.187         2,518         1,567         12' 0-1/4"         1,32           HF-D18 14x13         6,520         0.540         7,302         0.605         29,904         0.204         6,957         5,170         11' 9-1/4"         2,22           HF-D18 H 14x13         12,346         0.540         13,828         0.605         49,056         0.199         12,103         9,777         11' 5-3/4"         2,86           HF-D9 15x13         2,776         0.540         3,886         0.756         31,640         0.174         4,723         2,170         11' 10-1/4"         1,80           HF-D18 H 15x13 <t< td=""><td>HF-D18 12x13</td><td>7,555</td><td>0.540</td><td>8,462</td><td>0.605</td><td>34,608</td><td>0.179</td><td>7,539</td><td>6,844</td><td>11' 9-1/4"</td><td>2,133</td></t<>	HF-D18 12x13	7,555	0.540	8,462	0.605	34,608	0.179	7,539	6,844	11' 9-1/4"	2,133
HF-D18 13x13 7,004 0.540 7,844 0.605 32,088 0.192 7,235 5,926 11' 9-1/4" 2,18 HF-D9 14x13 2,368 0.540 3,315 0.756 28,000 0.171 4,085 1,969 11' 11-1/4" 1,70 HF-D12 14x13 1,916 0.540 2,146 0.605 13,776 0.187 2,518 1,567 12' 0-1/4" 1,32 HF-D18 14x13 6,520 0.540 7,302 0.605 29,904 0.204 6,957 5,170 11' 9-1/4" 2,22 HF-D18 H 14x13 12,346 0.540 13,828 0.605 49,056 0.199 12,103 9,777 11' 5-3/4" 2,86 HF-D9 15x13 2,776 0.540 3,886 0.756 31,640 0.174 4,723 2,170 11' 10-1/4" 1,80 HF-D18 15x13 6,092 0.540 6,823 0.605 28,056 0.216 6,725 4,547 11' 9-1/4" 2,27 HF-D18 H 15x13 11,612 0.540 13,005 0.605 46,032 0.212 11,723 8,658 11' 5-3/4" 2,93 HF-D9 16x13 2,659 0.540 3,723 0.756 29,680 0.186 4,673 1,955 11' 10-1/4" 1,84 HF-D18 16x13 5,713 0.540 6,399 0.605 26,376 0.229 6,519 4,031 11' 9-1/4" 2,32 HF-D18 H 16x13 10,949 0.540 11,593 0.605 40,992 0.237 11,069 6,911 11' 5-3/4" 3,06	HF-D9 13x13	2,498	0.540	3,497	0.756	29,960	0.159	4,132	2,223	11' 11-1/4"	1,664
HF-D9 14x13         2,368         0.540         3,315         0.756         28,000         0.171         4,085         1,969         11' 11-1/4"         1,70           HF-D12 14x13         1,916         0.540         2,146         0.605         13,776         0.187         2,518         1,567         12' 0-1/4"         1,32           HF-D18 14x13         6,520         0.540         7,302         0.605         29,904         0.204         6,957         5,170         11' 9-1/4"         2,22           HF-D18 H 14x13         12,346         0.540         13,828         0.605         49,056         0.199         12,103         9,777         11' 5-3/4"         2,86           HF-D9 15x13         2,776         0.540         3,886         0.756         31,640         0.174         4,723         2,170         11' 10-1/4"         1,80           HF-D18 I 5x13         6,092         0.540         6,823         0.605         28,056         0.216         6,725         4,547         11' 9-1/4"         2,93           HF-D18 H 15x13         11,612         0.540         13,005         0.605         46,032         0.212         11,723         8,658         11' 5-3/4"         2,93           HF-D18 H 16x13	HF-D12 13x13	2,045	0.540	2,290	0.605	14,784	0.174	2,585	1,788	12' 0-1/4"	1,294
HF-D12 14x13         1,916         0.540         2,146         0.605         13,776         0.187         2,518         1,567         12' 0-1/4"         1,32           HF-D18 14x13         6,520         0.540         7,302         0.605         29,904         0.204         6,957         5,170         11' 9-1/4"         2,22           HF-D18 H 14x13         12,346         0.540         13,828         0.605         49,056         0.199         12,103         9,777         11' 5-3/4"         2,86           HF-D9 15x13         2,776         0.540         3,886         0.756         31,640         0.174         4,723         2,170         11' 10-1/4"         1,80           HF-D18 H 5x13         6,092         0.540         6,823         0.605         28,056         0.216         6,725         4,547         11' 9-1/4"         2,27           HF-D18 H 15x13         11,612         0.540         13,005         0.605         46,032         0.212         11,723         8,658         11' 5-3/4"         2,93           HF-D9 16x13         2,659         0.540         3,723         0.756         29,680         0.186         4,673         1,955         11' 10-1/4"         1,84           HF-D18 H 16x13	HF-D18 13x13	7,004	0.540	7,844	0.605	32,088	0.192	7,235	5,926	11' 9-1/4"	2,181
HF-D18 14x13         6,520         0.540         7,302         0.605         29,904         0.204         6,957         5,170         11' 9-1/4"         2,22           HF-D18 H 14x13         12,346         0.540         13,828         0.605         49,056         0.199         12,103         9,777         11' 5-3/4"         2,86           HF-D9 15x13         2,776         0.540         3,886         0.756         31,640         0.174         4,723         2,170         11' 10-1/4"         1,80           HF-D18 15x13         6,092         0.540         6,823         0.605         28,056         0.216         6,725         4,547         11' 9-1/4"         2,27           HF-D18 H 15x13         11,612         0.540         13,005         0.605         46,032         0.212         11,723         8,658         11' 5-3/4"         2,93           HF-D9 16x13         2,659         0.540         3,723         0.756         29,680         0.186         4,673         1,955         11' 10-1/4"         1,84           HF-D18 H 16x13         5,713         0.540         6,399         0.605         26,376         0.229         6,519         4,031         11' 9-1/4"         2,32           HF-D18 H 17x13	HF-D9 14x13	2,368	0.540	3,315	0.756	28,000	0.171	4,085	1,969	11' 11-1/4"	1,700
HF-D18H 14x13         12,346         0.540         13,828         0.605         49,056         0.199         12,103         9,777         11' 5-3/4"         2,86           HF-D9 15x13         2,776         0.540         3,886         0.756         31,640         0.174         4,723         2,170         11' 10-1/4"         1,80           HF-D18 15x13         6,092         0.540         6,823         0.605         28,056         0.216         6,725         4,547         11' 9-1/4"         2,27           HF-D18 H 15x13         11,612         0.540         13,005         0.605         46,032         0.212         11,723         8,658         11' 5-3/4"         2,93           HF-D9 16x13         2,659         0.540         3,723         0.756         29,680         0.186         4,673         1,955         11' 10-1/4"         1,84           HF-D18 H 6x13         5,713         0.540         6,399         0.605         26,376         0.229         6,519         4,031         11' 9-1/4"         2,32           HF-D18 H 16x13         10,949         0.540         12,263         0.605         43,344         0.224         11,370         7,714         11' 5-3/4"         2,99           HF-D18 H 17x13	HF-D12 14x13	1,916	0.540	2,146	0.605	13,776	0.187	2,518	1,567	12' 0-1/4"	1,326
HF-D9 15x13         2,776         0.540         3,886         0.756         31,640         0.174         4,723         2,170         11' 10-1/4"         1,80           HF-D18 15x13         6,092         0.540         6,823         0.605         28,056         0.216         6,725         4,547         11' 9-1/4"         2,27           HF-D18 H 15x13         11,612         0.540         13,005         0.605         46,032         0.212         11,723         8,658         11' 5-3/4"         2,93           HF-D9 16x13         2,659         0.540         3,723         0.756         29,680         0.186         4,673         1,955         11' 10-1/4"         1,84           HF-D18 H6x13         5,713         0.540         6,399         0.605         26,376         0.229         6,519         4,031         11' 9-1/4"         2,32           HF-D18 H 16x13         10,949         0.540         12,263         0.605         43,344         0.224         11,370         7,714         11' 5-3/4"         2,99           HF-D18 H 17x13         10,351         0.540         11,593         0.605         40,992         0.237         11,069         6,911         11' 5-3/4"         3,06	HF-D18 14x13	6,520	0.540	7,302	0.605	29,904	0.204	6,957	5,170	11' 9-1/4"	2,229
HF-D18 15x13         6,092         0.540         6,823         0.605         28,056         0.216         6,725         4,547         11' 9-1/4"         2,27           HF-D18 H 15x13         11,612         0.540         13,005         0.605         46,032         0.212         11,723         8,658         11' 5-3/4"         2,93           HF-D9 16x13         2,659         0.540         3,723         0.756         29,680         0.186         4,673         1,955         11' 10-1/4"         1,84           HF-D18 H6x13         5,713         0.540         6,399         0.605         26,376         0.229         6,519         4,031         11' 9-1/4"         2,32           HF-D18 H 16x13         10,949         0.540         12,263         0.605         43,344         0.224         11,370         7,714         11' 5-3/4"         2,99           HF-D18 H 17x13         10,351         0.540         11,593         0.605         40,992         0.237         11,069         6,911         11' 5-3/4"         3,06	HF-D18 <b>H</b> 14x13	12,346	0.540	13,828	0.605	49,056	0.199	12,103	9,777	11' 5-3/4"	2,868
HF-D18H 15x13         11,612         0.540         13,005         0.605         46,032         0.212         11,723         8,658         11' 5-3/4"         2,93           HF-D9 16x13         2,659         0.540         3,723         0.756         29,680         0.186         4,673         1,955         11' 10-1/4"         1,84           HF-D18 16x13         5,713         0.540         6,399         0.605         26,376         0.229         6,519         4,031         11' 9-1/4"         2,32           HF-D18 H 16x13         10,949         0.540         12,263         0.605         43,344         0.224         11,370         7,714         11' 5-3/4"         2,99           HF-D18 H 17x13         10,351         0.540         11,593         0.605         40,992         0.237         11,069         6,911         11' 5-3/4"         3,06	HF-D9 15x13	2,776	0.540	3,886	0.756	31,640	0.174	4,723	2,170	11' 10-1/4"	1,808
HF-D9 16x13       2,659       0.540       3,723       0.756       29,680       0.186       4,673       1,955       11' 10-1/4"       1,84         HF-D18 16x13       5,713       0.540       6,399       0.605       26,376       0.229       6,519       4,031       11' 9-1/4"       2,32         HF-D18 H 16x13       10,949       0.540       12,263       0.605       43,344       0.224       11,370       7,714       11' 5-3/4"       2,99         HF-D18 H 17x13       10,351       0.540       11,593       0.605       40,992       0.237       11,069       6,911       11' 5-3/4"       3,06	HF-D18 15x13	6,092	0.540	6,823	0.605	28,056	0.216	6,725	4,547	11' 9-1/4"	2,277
HF-D18 16x13     5,713     0.540     6,399     0.605     26,376     0.229     6,519     4,031     11' 9-1/4"     2,32       HF-D18 H 16x13     10,949     0.540     12,263     0.605     43,344     0.224     11,370     7,714     11' 5-3/4"     2,99       HF-D18 H 17x13     10,351     0.540     11,593     0.605     40,992     0.237     11,069     6,911     11' 5-3/4"     3,06	HF-D18 <b>H</b> 15x13	11,612	0.540	13,005	0.605	46,032	0.212	11,723	8,658	11' 5-3/4"	2,933
HF-D18H 16x13     10,949     0.540     12,263     0.605     43,344     0.224     11,370     7,714     11' 5-3/4"     2,99       HF-D18H 17x13     10,351     0.540     11,593     0.605     40,992     0.237     11,069     6,911     11' 5-3/4"     3,06	HF-D9 16x13	2,659	0.540	3,723	0.756	29,680	0.186	4,673	1,955	11' 10-1/4"	1,847
HF-D18 <b>H</b> 17x13 10,351 0.540 11,593 0.605 40,992 0.237 11,069 6,911 11'5-3/4" 3,06	HF-D18 16x13	5,713	0.540	6,399	0.605	26,376	0.229	6,519	4,031	11' 9-1/4"	2,325
HF-D18 <b>H</b> 17x13 10,351 0.540 11,593 0.605 40,992 0.237 11,069 6,911 11'5-3/4" 3,06	HF-D18 <b>H</b> 16x13	10,949	0.540	12,263	0.605	43,344	0.224	11,370	7,714	11' 5-3/4"	2,999
											3,064
		•							*		3,129
HF-D18 <b>H</b> 19x13 9,314 0.540 10,432 0.605 36,624 0.261 10,469 5,632 11' 5-3/4" 3,19		*	0.540					·		11' 5-3/4"	3,194
		*									3,259
		*						·			3,325
		•							*		3,390
		*		·							3,455

<sup>1.</sup> The values in this table are Allowable Stress Design (ASD) excluding a 1.33 stress increase and pertain to pinned base installation on 2500 psi concrete or nut & washers with high strength non-shrink grout

 $V_R = (V_{Dsn}/2) + (V_{Mult} \times Max P_{Dsn})$ 

Where:  $V_{Dsn}$  = Design Shear Load (lbs)

 $P_{Dsn}^{\text{SSI}}$  = Design Aggregate Point Load at the beam mid-span (lbs)

6. The Uplift on a column is calculated with the following equation:

 $Uplift = [(V_{Dsn} x MF Height) / W_{CL}] - (Min P_{Dsn} / 2)$ 

 $\textbf{Where:} \ \textbf{V}_{Dsn} = \textbf{Design Shear Load (lbs)}$ 

MF Height = Dimension from top of concrete to top of the wall framing (ft)

 $W_{CL}$  = Inside steel to steel dimension (Win) + the column depth (ft)

<sup>2.</sup> Drift at the allowable shear load is within the code allowed limits for R=6.5,  $C_d$ =4.0,  $\Delta_a$  = 0.025h

<sup>3.</sup> Pu is the allowable aggregate point load applied at the beam mid-span in combination with the vertical load produced by overturning at the allowable shear load.

<sup>4.</sup> The table value for Resultatnt Shear (V<sub>R</sub>) assumes the Allowable Shear (V) in combination with the Maximum Additional Vertical Load (Pu).

<sup>5.</sup> The design Resultant Shear (V<sub>R</sub>) from combined horizontal and compression forces on a column are solved with the following equation:

 $P_{Dsn} = \text{Design Aggregate Point Load at the beam mid-span (lbs)} \\ 7. \text{ h1 is the dimension from top of concrete to bottom of the lowest point in the header assembly assuming installation on nuts and washers with the Base Plate plate elevated 1-1/2" above finish concrete <math display="block"> P_{Dsn} = P_{Dsn$ 

Node    Seismic   Wind   Maximum Add'  Vertical   Shear Multiplier   Column Base Reactions   National Properties   Column Vertical   Resultant Shear V (Resultant	Approx Weight (lbs) 1,449 1,481 1,513 1,563 1,181 1,876
Allowable Shear V ⊕ R= 6.5 (lbs)         Drift² ⊕ Allow V (in)         Allowable Shear V (lbs)         Drift² ⊕ Allow V (in)         Load Pu (lbs)         Column V Mult         Resultant Shear⁵ V R ⊕ Allow V when Pu = 0 (lbs)         Height (ft-in)           HF-D9 6x14         2,911         0.728         4,075         1.019         45,640         0.073         3,490         5,458         13' 0-1/4"           HF-D9 7x14         2,673         0.728         3,742         0.722         40,600         0.085         3,429         4,406         13' 0-1/4"           HF-D9 8x14         2,461         0.728         3,445         0.722         36,680         0.096         3,372         3,623         13' 0-1/4"           HF-D9H 8x14         2,979         0.728         4,171         0.722         43,960         0.089         3,853         4,386         12' 11-1/4"           HF-D12 8x14         2,667         0.728         2,987         0.578         15,624         0.103         2,311         3,817         13' 0-1/4"           HF-D18 8x14         6,071         0.728         6,800         0.578         34,440         0.126         5,674         8,290         12' 11-1/4"           HF-D19 9x14         2,798         0.728         3,917         0.722	1,449 1,481 1,513 1,563 1,181
HF-D9 7x14         2,673         0.728         3,742         0.722         40,600         0.085         3,429         4,406         13' 0-1/4"           HF-D9 8x14         2,461         0.728         3,445         0.722         36,680         0.096         3,372         3,623         13' 0-1/4"           HF-D9 M 8x14         2,979         0.728         4,171         0.722         43,960         0.089         3,853         4,386         12' 11-1/4"           HF-D12 8x14         2,667         0.728         2,987         0.578         15,624         0.103         2,311         3,817         13' 0-1/4"           HF-D18 8x14         6,071         0.728         6,800         0.578         34,440         0.126         5,674         8,290         12' 11-3/4"           HF-D9 9x14         2,798         0.728         3,917         0.722         40,320         0.100         3,840         3,723         12' 11-1/4"           HF-D12 9x14         2,457         0.728         2,752         0.578         15,792         0.116         2,336         3,188         13' 0-1/4"           HF-D18 9x14         8,615         0.728         9,649         0.578         43,848         0.129         7,745         10,6	1,481 1,513 1,563 1,181
HF-D9 8x14         2,461         0.728         3,445         0.722         36,680         0.096         3,372         3,623         13' 0-1/4"           HF-D9H 8x14         2,979         0.728         4,171         0.722         43,960         0.089         3,853         4,386         12' 11-1/4"           HF-D12 8x14         2,667         0.728         2,987         0.578         15,624         0.103         2,311         3,817         13' 0-1/4"           HF-D18 8x14         6,071         0.728         6,800         0.578         34,440         0.126         5,674         8,290         12' 11-3/4"           HF-D9 9x14         2,798         0.728         3,917         0.722         40,320         0.100         3,840         3,723         12' 11-1/4"           HF-D12 9x14         2,457         0.728         2,752         0.578         15,792         0.116         2,336         3,188         13' 0-1/4"           HF-D18 9x14         8,615         0.728         9,649         0.578         43,848         0.129         7,745         10,694         12' 9-1/4"           HF-D9 10x14         2,632         0.728         3,685         0.722         36,960         0.111         3,799         3,1	1,513 1,563 1,181
HF-D9H 8x14 2,979 0.728 4,171 0.722 43,960 0.089 3,853 4,386 12' 11-1/4" HF-D12 8x14 2,667 0.728 2,987 0.578 15,624 0.103 2,311 3,817 13' 0-1/4" HF-D18 8x14 6,071 0.728 6,800 0.578 34,440 0.126 5,674 8,290 12' 11-3/4" HF-D9 9x14 2,798 0.728 3,917 0.722 40,320 0.100 3,840 3,723 12' 11-1/4" HF-D12 9x14 2,457 0.728 2,752 0.578 15,792 0.116 2,336 3,188 13' 0-1/4" HF-D18 9x14 8,615 0.728 9,649 0.578 43,848 0.129 7,745 10,694 12' 9-1/4" HF-D9 10x14 2,632 0.728 3,685 0.722 36,960 0.111 3,799 3,194 12' 11-1/4" HF-D12 10x14 2,271 0.728 2,544 0.578 15,792 0.127 2,350 2,692 13' 0-1/4" HF-D18 10x14 7,923 0.728 8,874 0.578 40,152 0.141 7,396 9,020 12' 9-1/4" HF-D9 11x14 2,482 0.728 3,475 0.722 34,160 0.122 3,776 2,766 12' 11-1/4"	1,563 1,181
HF-D12 8x14         2,667         0.728         2,987         0.578         15,624         0.103         2,311         3,817         13' 0-1/4"           HF-D18 8x14         6,071         0.728         6,800         0.578         34,440         0.126         5,674         8,290         12' 11-3/4"           HF-D9 9x14         2,798         0.728         3,917         0.722         40,320         0.100         3,840         3,723         12' 11-1/4"           HF-D12 9x14         2,457         0.728         2,752         0.578         15,792         0.116         2,336         3,188         13' 0-1/4"           HF-D18 9x14         8,615         0.728         9,649         0.578         43,848         0.129         7,745         10,694         12' 9-1/4"           HF-D9 10x14         2,632         0.728         3,685         0.722         36,960         0.111         3,799         3,194         12' 11-1/4"           HF-D12 10x14         2,271         0.728         2,544         0.578         15,792         0.127         2,350         2,692         13' 0-1/4"           HF-D18 10x14         7,923         0.728         8,874         0.578         40,152         0.141         7,396	1,181
HF-D18 8x14         6,071         0.728         6,800         0.578         34,440         0.126         5,674         8,290         12' 11-3/4"           HF-D9 9x14         2,798         0.728         3,917         0.722         40,320         0.100         3,840         3,723         12' 11-1/4"           HF-D12 9x14         2,457         0.728         2,752         0.578         15,792         0.116         2,336         3,188         13' 0-1/4"           HF-D18 9x14         8,615         0.728         9,649         0.578         43,848         0.129         7,745         10,694         12' 9-1/4"           HF-D9 10x14         2,632         0.728         3,685         0.722         36,960         0.111         3,799         3,194         12' 11-1/4"           HF-D12 10x14         2,271         0.728         2,544         0.578         15,792         0.127         2,350         2,692         13' 0-1/4"           HF-D18 10x14         7,923         0.728         8,874         0.578         40,152         0.141         7,396         9,020         12' 9-1/4"           HF-D9 11x14         2,482         0.728         3,475         0.722         34,160         0.122         3,776	-
HF-D9 9x14         2,798         0.728         3,917         0.722         40,320         0.100         3,840         3,723         12' 11-1/4"           HF-D12 9x14         2,457         0.728         2,752         0.578         15,792         0.116         2,336         3,188         13' 0-1/4"           HF-D18 9x14         8,615         0.728         9,649         0.578         43,848         0.129         7,745         10,694         12' 9-1/4"           HF-D9 10x14         2,632         0.728         3,685         0.722         36,960         0.111         3,799         3,194         12' 11-1/4"           HF-D12 10x14         2,271         0.728         2,544         0.578         15,792         0.127         2,350         2,692         13' 0-1/4"           HF-D18 10x14         7,923         0.728         8,874         0.578         40,152         0.141         7,396         9,020         12' 9-1/4"           HF-D9 11x14         2,482         0.728         3,475         0.722         34,160         0.122         3,776         2,766         12' 11-1/4"	1,876
HF-D12 9x14         2,457         0.728         2,752         0.578         15,792         0.116         2,336         3,188         13' 0-1/4"           HF-D18 9x14         8,615         0.728         9,649         0.578         43,848         0.129         7,745         10,694         12' 9-1/4"           HF-D9 10x14         2,632         0.728         3,685         0.722         36,960         0.111         3,799         3,194         12' 11-1/4"           HF-D12 10x14         2,271         0.728         2,544         0.578         15,792         0.127         2,350         2,692         13' 0-1/4"           HF-D18 10x14         7,923         0.728         8,874         0.578         40,152         0.141         7,396         9,020         12' 9-1/4"           HF-D9 11x14         2,482         0.728         3,475         0.722         34,160         0.122         3,776         2,766         12' 11-1/4"	
HF-D18 9x14         8,615         0.728         9,649         0.578         43,848         0.129         7,745         10,694         12' 9-1/4"           HF-D9 10x14         2,632         0.728         3,685         0.722         36,960         0.111         3,799         3,194         12' 11-1/4"           HF-D12 10x14         2,271         0.728         2,544         0.578         15,792         0.127         2,350         2,692         13' 0-1/4"           HF-D18 10x14         7,923         0.728         8,874         0.578         40,152         0.141         7,396         9,020         12' 9-1/4"           HF-D9 11x14         2,482         0.728         3,475         0.722         34,160         0.122         3,776         2,766         12' 11-1/4"	1,599
HF-D9 10x14         2,632         0.728         3,685         0.722         36,960         0.111         3,799         3,194         12' 11-1/4"           HF-D12 10x14         2,271         0.728         2,544         0.578         15,792         0.127         2,350         2,692         13' 0-1/4"           HF-D18 10x14         7,923         0.728         8,874         0.578         40,152         0.141         7,396         9,020         12' 9-1/4"           HF-D9 11x14         2,482         0.728         3,475         0.722         34,160         0.122         3,776         2,766         12' 11-1/4"	1,214
HF-D12 10x14     2,271     0.728     2,544     0.578     15,792     0.127     2,350     2,692     13' 0-1/4"       HF-D18 10x14     7,923     0.728     8,874     0.578     40,152     0.141     7,396     9,020     12' 9-1/4"       HF-D9 11x14     2,482     0.728     3,475     0.722     34,160     0.122     3,776     2,766     12' 11-1/4"	2,066
HF-D18 10x14     7,923     0.728     8,874     0.578     40,152     0.141     7,396     9,020     12' 9-1/4"       HF-D9 11x14     2,482     0.728     3,475     0.722     34,160     0.122     3,776     2,766     12' 11-1/4"	1,635
HF-D9 11x14 2,482 0.728 3,475 0.722 34,160 0.122 3,776 2,766 12' 11-1/4"	1,246
	2,114
HF-D12 11x14   2 109   0.728   2.362   0.578   15.624   0.138   2.360   2.297   13\ \text{0-1}\ \text{4}\ \text{0-1}\ \text{5}\ \text{2}\ \text{3}\ \text{6}\ \text{2}\ \text{3}\ \text{6}\ \text{2}\ \text{3}\ \text{6}\ \text{2}\ \text{3}\ \text{6}\ \text{3}\ \text{6}\ \text{2}\ \text{3}\ \text{6}\ \text{2}\ \text{3}\ \text{6}\ \text{2}\ \text{3}\ \text{6}\ \text{2}\ \text{3}\ \text{6}\ \text{3}\ \text{6}\ \text{2}\ \text{3}\ \text{6}\ \text{6}\ \text{3}\ \text{6}\ \text{6}	1,670
2,201 10 0114	1,278
HF-D18 11x14         7,313         0.728         8,191         0.578         36,960         0.152         7,077         7,688         12' 9-1/4"	2,162
HF-D9 12x14 2,346 0.728 3,284 0.722 31,640 0.134 3,739 2,417 12' 11-1/4"	1,706
HF-D12 12x14         1,965         0.728         2,201         0.578         15,288         0.149         2,364         1,989         13' 0-1/4"	1,310
HF-D18 12x14         6,772         0.728         7,585         0.578         34,272         0.164         6,796         6,616         12' 9-1/4"	2,210
HF-D9 13x14 2,223 0.728 3,112 0.722 29,680 0.144 3,712 2,136 12' 11-1/4"	1,741
HF-D12 13x14         1,840         0.728         2,061         0.578         14,616         0.161         2,346         1,734         13' 0-1/4"	1,342
HF-D18 13x14         6,294         0.728         7,049         0.578         31,752         0.176         6,531         5,739         12' 9-1/4"	2,258
HF-D9 14x14 2,111 0.728 2,955 0.722 27,720 0.156 3,678 1,895 12' 11-1/4"	1,777
HF-D12 14x14 1,726 0.728 1,933 0.578 13,608 0.172 2,280 1,520 13' 0-1/4"	1,374
HF-D18 14x14         5,873         0.728         6,578         0.578         29,736         0.187         6,318         5,022         12' 9-1/4"	2,306
HF-D18 <b>H</b> 14x14	2,968
HF-D9 15x14 2,460 0.728 3,444 0.722 31,080 0.158 4,202 2,069 12' 10-1/4"	1,885
HF-D18 15x14 5,497 0.728 6,157 0.578 27,888 0.199 6,112 4,426 12' 9-1/4"	2,354
HF-D18 <b>H</b> 15x14	3,033
HF-D9 16x14 2,360 0.728 3,304 0.722 29,400 0.170 4,205 1,875 12' 10-1/4"	1,924
HF-D18 16x14         5,163         0.728         5,783         0.578         26,208         0.210         5,927         3,931         12' 9-1/4"	2,402
HF-D18 <b>H</b> 16x14 9,810 0.728 10,987 0.578 43,008 0.206 10,272 7,453 12' 5-3/4"	3,098
HF-D18 <b>H</b> 17x14 9,291 0.728 10,406 0.578 40,656 0.217 10,006 6,690 12' 5-3/4"	3,163
HF-D18 <b>H</b> 18x14 8,818 0.728 9,876 0.578 38,304 0.229 9,729 6,033 12' 5-3/4"	3,228
HF-D18 <b>H</b> 19x14 8,388 0.728 9,395 0.578 36,456 0.240 9,515 5,471 12' 5-3/4"	3,293
HF-D18 <b>H</b> 20x14 7,994 0.728 8,953 0.578 34,608 0.252 9,296 4,975 12' 5-3/4"	3,359
HF-D18 <b>H</b> 21x14 7,632 0.728 8,548 0.578 33,096 0.264 9,116 4,547 12' 5-3/4"	5,559
HF-D18 <b>H</b> 22x14 7,300 0.728 8,176 0.578 31,584 0.276 8,936 4,165 12' 5-3/4"	3,424
HF-D18 <b>H</b> 23x14 6,993 0.728 7,832 0.578 30,072 0.287 8,740 3,830 12' 5-3/4"	

<sup>1.</sup> The values in this table are Allowable Stress Design (ASD) excluding a 1.33 stress increase and pertain to pinned base installation on 2500 psi concrete or nut & washers with high strength non-shrink grout

 $\ensuremath{\mathsf{6}}.$  The Uplift on a column is calculated with the following equation:

 $V_R = (V_{Dsn}/2) + (V_{Mult} \times Max P_{Dsn})$ 

**Where:**  $V_{Dsn} = Design Shear Load (lbs)$  $\mbox{Uplift} = [(\mbox{V}_{\mbox{Dsn}} \mbox{ x MF Height}) \mbox{/ W}_{\mbox{CL}}] \mbox{ - (Min P}_{\mbox{Dsn}} \mbox{/ 2})$ 

MF Height = Dimension from top of concrete to top of the wall framing (ft)

 $P_{Dsn}$  = Design Aggregate Point Load at the beam mid-span (lbs)

W<sub>Cl</sub> = Inside steel to steel dimension (Win) + the column depth (ft)

 $P_{Dsn}^{--}$  = Design Aggregate Point Load at the beam mid-span (lbs)

<sup>2.</sup> Drift at the allowable shear load is within the code allowed limits for R=6.5,  $C_d$ =4.0,  $\Delta_a$  = 0.025h

<sup>3.</sup> Pu is the allowable aggregate point load applied at the beam mid-span in combination with the vertical load produced by overturning at the allowable shear load.

<sup>4.</sup> The table value for Resultatnt Shear (V<sub>R</sub>) assumes the Allowable Shear (V) in combination with the Maximum Additional Vertical Load (Pu).

<sup>5.</sup> The design Resultant Shear (V<sub>R</sub>) from combined horizontal and compression forces on a column are solved with the following equation:  $V_{Dsn}$  = Design Shear Load (lbs)

<sup>7.</sup> h1 is the dimension from top of concrete to bottom of the lowest point in the header assembly assuming installation on nuts and washers with the Base Plate plate elevated 1-1/2" above finish concrete

### HARDY A FRAMES.

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Table 1.15 - 15' 1" Nominal Wall Heights Net Wall Ht = 15' 0-3/4"										0-3/4"
	Seis	mic	Wi	ind	Maximum Add'l	Shear Multiplier <sup>4</sup>	Column Base I	Reactions	h1 <sup>7</sup>	
Model	Allowable <sup>1</sup> Shear V @ R= 6.5 (lbs)	Drift <sup>2</sup> @ Allow V (in)	Allowable Shear V (lbs)	Drift <sup>2</sup> @ Allow V (in)	Vertical <sup>3</sup> Load Pu (lbs)	@ Compression Column V <sub>Mult</sub>	Resultant Shear <sup>5</sup> VR @ Allow V & Max Pu (lbs)	Uplift <sup>6</sup> @ Allow V when Pu = 0 (lbs)	Opening Height (ft-in)	Approx Weight (lbs)
HF-D18 8x15	5,489	0.781	6,148	0.875	34,104	0.117	5,158	8,042	13' 11-3/4"	1,953
HF-D18 10x15	7,106	0.781	7,959	0.875	39,648	0.130	6,676	8,685	13' 9-1/4"	2,191
HF-D18 12x15	6,108	0.781	6,841	0.875	33,936	0.151	6,174	6,402	13' 9-1/4"	2,287
HF-D18 <b>H</b> 14x15	9,884	0.781	11,070	0.875	47,880	0.168	9,827	9,060	13' 5-3/4"	3,067
HF-D18 <b>H</b> 16x15	8,842	0.781	9,903	0.875	42,504	0.190	9,316	7,205	13' 5-3/4"	3,197
HF-D18 <b>H</b> 18x15	7,975	0.781	8,932	0.875	38,136	0.211	8,875	5,853	13' 5-3/4"	3,328
HF-D18 <b>H</b> 20x15	7,250	0.781	8,120	0.875	34,440	0.233	8,495	4,842	13' 5-3/4"	3,458
HF-D18 <b>H</b> 22x15	6,636	0.781	7,432	0.875	31,248	0.255	8,157	4,065	13' 5-3/4"	3,588
				Table	1.16 - 16' 1"	Nominal Wall	Heights	Net Wal	I Ht = 16'	0-3/4"
HF-D18 8x16	4,991	0.834	5,590	0.934	33,768	0.108	4,718	7,808	14' 11-3/4"	2,031
HF-D18 10x16	6,407	0.834	7,176	0.934	36,624	0.120	5,878	8,364	14' 9-1/4"	2,269
HF-D18 12x16	5,539	0.834	6,204	0.934	33,600	0.140	5,634	6,201	14' 9-1/4"	2,365
HF-D18 <b>H</b> 14x16	8,924	0.834	9,995	0.934	47,376	0.156	8,937	8,732	14' 5-3/4"	3,166
HF-D18 <b>H</b> 16x16	8,015	0.834	8,977	0.934	42,000	0.176	8,487	6,978	14' 5-3/4"	3,296
HF-D18 <b>H</b> 18x16	7,252	0.834	8,122	0.934	37,800	0.196	8,123	5,685	14' 5-3/4"	3,427
HF-D18 <b>H</b> 20x16	6,611	0.834	7,404	0.934	34,104	0.216	7,784	4,714	14' 5-3/4"	3,557
HF-D18 <b>H</b> 22x16	6,065	0.834	6,793	0.934	31,080	0.237	7,504	3,964	14' 5-3/4"	3,688
				Table	1.17 - 17' 1"	Nominal Wall	Heights	Net Wal	I Ht = 17'	0-3/4"
HF-D18 8x17	4,558	0.887	5,105	0.993	33,432	0.101	4,335	7,587	15' 11-3/4"	2,108
HF-D18 10x17	5,810	0.887	6,507	0.993	29,736	0.112	4,922	8,063	15' 9-1/4"	2,346
HF-D18 12x17	5,046	0.887	5,652	0.993	31,080	0.131	4,991	6,007	15' 9-1/4"	2,442
HF-D18 <b>H</b> 14x17	8,097	0.887	9,069	0.993	46,872	0.144	8,155	8,431	15' 5-3/4"	3,265
HF-D18 <b>H</b> 16x17	7,300	0.887	8,176	0.993	41,664	0.163	7,784	6,763	15' 5-3/4"	3,396
HF-D18 <b>H</b> 18x17	6,626	0.887	7,421	0.993	37,464	0.183	7,473	5,525	15' 5-3/4"	3,526
HF-D18 <b>H</b> 20x17	6,054	0.887	6,780	0.993	33,936	0.202	7,185	4,594	15' 5-3/4"	3,657
HF-D18 <b>H</b> 22x17	5,566	0.887	6,234	0.993	30,912	0.221	6,932	3,871	15' 5-3/4"	3,787

<sup>1.</sup> The values in this table are Allowable Stress Design (ASD) excluding a 1.33 stress increase and pertain to pinned base installation on 2500 psi concrete or nut & washers with high strength non-shrink grout

$$V_R = (V_{Dsn}/2) + (V_{Mult} \times Max P_{Dsn})$$

Where:  $V_{Dsn} = Design Shear Load (lbs)$ 

P<sub>Dsn</sub> = Design Aggregate Point Load at the beam mid-span (lbs)

6. The Uplift on a column is calculated with the following equation:

Uplift = 
$$[(V_{Dsn} \times MF \text{ Height}) / W_{CL}] - (Min P_{Dsn} / 2)$$

Where: V<sub>Dsn</sub> = Design Shear Load (lbs)

MF Height = Dimension from top of concrete to top of the wall framing (ft)

 $W_{Cl}$  = Inside steel to steel dimension (Win) + the column depth (ft)

P<sub>Dsn</sub> = Design Aggregate Point Load at the beam mid-span (lbs)

7. h1 is the dimension from top of concrete to bottom of the lowest point in the header assembly assuming installation on nuts and washers with the Base Plate plate elevated 1-1/2" above finish concrete

<sup>2.</sup> Drift at the allowable shear load is within the code allowed limits for R=6.5,  $C_d$ =4.0,  $\Delta_a$  = 0.025h

<sup>3.</sup> Pu is the allowable aggregate point load applied at the beam mid-span in combination with the vertical load produced by overturning at the allowable shear load.

<sup>4.</sup> The table value for Resultatnt Shear (V<sub>R</sub>) assumes the Allowable Shear (V) in combination with the Maximum Additional Vertical Load (Pu).

<sup>5.</sup> The design Resultant Shear  $(V_R)$  from combined horizontal and compression forces on a column are solved with the following equation:

	Table 1.18 - 18 1" Nominal Wall Heights Net Wall Ht = 18' 0-3/4"											
	Seis	smic	w	ind	Maximum Add'l	Shear Multiplier <sup>4</sup>	Column Base	Reactions	h1 <sup>7</sup>	Approx		
Model	Allowable <sup>1</sup> Shear V @ R= 6.5 (lbs)	Drift <sup>2</sup> @ Allow V (in)	Allowable Shear V (lbs)	Drift <sup>2</sup> @ Allow V (in)	Vertical <sup>3</sup> Load Pu (lbs)	@ Compression Column V <sub>Mult</sub>	Resultant Shear <sup>5</sup> VR @ Allow V & Max Pu (lbs)	Uplift <sup>6</sup> @ Allow V when Pu = 0 (lbs)	Opening Height (ft-in)	Weight (lbs)		
HF-D18 8x18	4,182	0.940	4,684	1.053	31,920	0.095	3,932	7,373	16' 11-3/4"	2,185		
HF-D18 10x18	5,293	0.940	5,928	1.053	23,688	0.105	4,158	7,788	16' 9-1/4"	2,423		
HF-D18 12x18	4,619	0.940	5,173	1.053	26,544	0.123	4,284	5,833	16' 9-1/4"	2,519		
HF-D18 <b>H</b> 14x18	7,383	0.940	8,269	1.053	43,680	0.135	7,266	8,150	16' 5-3/4"	3,365		
HF-D18 <b>H</b> 16x18	6,678	0.940	7,479	1.053	41,328	0.153	7,170	6,556	16' 5-3/4"	3,495		
HF-D18 <b>H</b> 18x18	6,078	0.940	6,807	1.053	37,128	0.171	6,893	5,371	16' 5-3/4"	3,626		
HF-D18 <b>H</b> 20x18	5,567	0.940	6,235	1.053	33,768	0.189	6,656	4,480	16' 5-3/4"	3,756		
HF-D18 <b>H</b> 22x18	5,129	0.940	5,744	1.053	30,744	0.207	6,435	3,783	16' 5-3/4"	3,886		
				Table 1.19	9 - 19' 1" Non	ninal Wall Heig	hts	Net Wall	Ht = 19' 0-	-3/4 <sup>11</sup>		
HF-D18 8x19	3,851	0.993	4,313	1.112	27,048	0.089	3,393	7,179	17' 11-3/4"	2,263		
HF-D18 10x19	4,842	0.993	5,423	1.112	18,816	0.099	3,546	7,527	17' 9-1/4"	2,500		
HF-D18 12x19	4,245	0.993	4,754	1.112	22,512	0.115	3,695	5,658	17' 9-1/4"	2,596		
HF-D18 <b>H</b> 14x19	6,758	0.993	7,569	1.112	37,968	0.126	6,292	7,882	17' 5-3/4"	3,464		
HF-D18 <b>H</b> 16x19	6,133	0.993	6,869	1.112	37,464	0.144	6,332	6,362	17' 5-3/4"	3,594		
HF-D18 <b>H</b> 18x19	5,598	0.993	6,270	1.112	36,624	0.160	6,364	5,230	17' 5-3/4"	3,725		
HF-D18 <b>H</b> 20x19	5,139	0.993	5,756	1.112	33,432	0.178	6,172	4,366	17' 5-3/4"	3,855		
HF-D18 <b>H</b> 22x19	4,744	0.993	5,313	1.112	30,576	0.195	5,983	3,696	17' 5-3/4"	3,985		
				<b>Table 1.2</b> 0	0 - <mark>20' 1" No</mark> n	ninal Wall Heig	hts	Net Wall	Ht = 20' 0-	3/4"		
HF-D18 8x20	3,559	1.046	3,986	1.172	22,680	0.084	2,939	6,984	18' 11-3/4"	2,340		
HF-D18 10x20	4,447	1.046	4,981	1.172	14,784	0.094	3,064	7,286	18' 9-1/4"	2,578		
HF-D18 12x20	3,915	1.046	4,385	1.172	18,480	0.109	3,180	5,504	18' 9-1/4"	2,674		
HF-D18 <b>H</b> 14x20	6,211	1.046	6,956	1.172	33,096	0.119	5,495	7,627	18' 5-3/4"	3,563		
HF-D18 <b>H</b> 16x20	5,653	1.046	6,331	1.172	33,096	0.135	5,545	6,181	18' 5-3/4"	3,694		
HF-D18 <b>H</b> 18x20	5,173	1.046	5,794	1.172	32,424	0.151	5,556	5,089	18' 5-3/4"	3,824		
HF-D18 <b>H</b> 20x20	4,759	1.046	5,330	1.172	31,752	0.167	5,600	4,259	18' 5-3/4"	3,954		
HF-D18 <b>H</b> 22x20	4,402	1.046	4,930	1.172	30,408	0.184	5,588	3,616	18' 5-3/4"	4,085		

<sup>1.</sup> The values in this table are Allowable Stress Design (ASD) excluding a 1.33 stress increase and pertain to pinned base installation on 2500 psi concrete or nut & washers with high strength non-shrink grout

5. The design Resultant Shear (V<sub>R</sub>) from combined horizontal and compression forces on a column are solved with the following equation:

$$V_R = (V_{Dsn}/2) + (V_{Mult} \times Max P_{Dsn})$$

Where:  $V_{Dsn}$  = Design Shear Load (lbs)

P<sub>Dsn</sub> = Design Aggregate Point Load at the beam mid-span (lbs)

6. The Uplift on a column is calculated with the following equation:

 $Uplift = [(V_{Dsn} x MF Height) / W_{CL}] - (Min P_{Dsn} / 2)$ 

Where:  $V_{Dsn} = Design Shear Load (lbs)$ 

MF Height = Dimension from top of concrete to top of the wall framing (ft)

 $W_{CL} =$ Inside steel to steel dimension (Win) + the column depth (ft)

 $P_{Dsn}^{CL}$  = Design Aggregate Point Load at the beam mid-span (lbs) 7. h1 is the dimension from top of concrete to bottom of the lowest point in the header assembly assuming installation on nuts and washers with the Base Plate plate elevated 1-1/2" above finish concrete

<sup>2.</sup> Drift at the allowable shear load is within the code allowed limits for R=6.5,  $C_d$ =4.0,  $\Delta_a$  = 0.025h

<sup>3.</sup> Pu is the allowable aggregate point load applied at the beam mid-span in combination with the vertical load produced by overturning at the allowable shear load.

<sup>4.</sup> The table value for Resultatnt Shear (V<sub>R</sub>) assumes the Allowable Shear (V) in combination with the Maximum Additional Vertical Load (Pu).



### Installer

### **How to Quote a Hardy Frame® Moment Frame:**

Locate the model number call-out on Foundation and Framing plans.

**Note:** One Moment Frame consists of two columns and a header. Be careful not to double the count by confusing Moment Frame columns with individual Hardy Frame<sup>®</sup> Panels.

- Call your material supplier or Hardy Frames with the following information for a job quote:
  - Job name and location
  - Quantity and model number
  - Project accessibility for trucks to deliver
  - Jobsite access for installing the Frame
- · Check anchorage edge dimensions to determine wood framing requirements
- Check job conditions for accessibility and placement of pre-assembled Frames with a forklift or crane.

### **Ordering a Hardy Frame® Moment Frame:**

- Prior to fabrication a Moment Frame "Dimension Sheet" will be provided indicating standard inside and outside widths and heights. This is a working document that allows the contractor to adjust dimensions, request changes to wood nailer sizes or customize nailer applications.
- Template Kits are provided when purchasing Moment Frames. They are in stock and can ship within one to two business days to expedite pouring concrete.
- Allow 15 to 20 working days for delivery of Moment Frame

### **Installing Template Kits and Hold Down Anchors:**

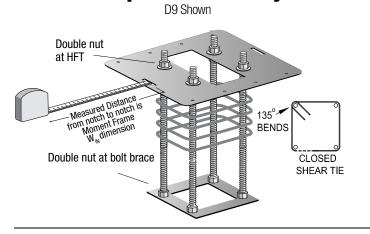
- Assemble the Kit as shown in Hardy Frame Typical Installation Details and the illustrations provided on the Template and Template Kit box labels
- Locate one Template Kit assembly at each of the column locations
- Be sure to place the Templates so that the edges with the slotted holes are oriented in the same direction as the Moment Frame wall line
- Measure the interior slot to slot distance to be the same as the "W<sub>in</sub>" (inside steel to steel) dimension for the Frame being ordered.
- Set the top of all hold down bolts at 4-1/4 inches (minimum) to 6 inches (maximum) above top of concrete. Be sure that the embed end is extending the required distance into the footing below.
- Pour concrete and check that the assembly did not move out of alignment prior to the concrete hardening.

### **Installing the Hardy Frame® Moment Frame:**

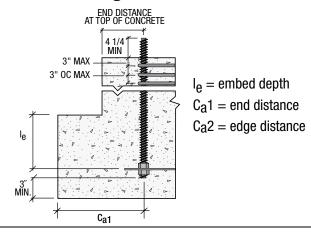
- Install one nut with one washer above on all hold down anchors. Set the top of washer at approximately 1-1/2 inches above top of concrete.
- Unload the Moment Frame by lifting with forks or a crane at the header.
- Install the Moment Frame on the hold downs so that the base plates seat firmly on the washers and nuts below.
- Install one washer in contact with the base plate and one nut above it on all hold down bolts.
- Level the Frame and make minor height adjustments by raising or lowering the double nut connection at the base plates, then secure the connection until all nuts are "snug-tight". For installations on nuts and washers with high strength non-shrink grout, third party inspection may be required.
- Install wood framing above per the specified installation details and make connections to transfer shear loads. If the specified connection will be concealed when framing is installed above, be sure to get inspections while still visible.

### HARDY A FRAMES

### **Template Kit Assembly**



### **Anchorage Dimensions**



- **Moment Frame Template Kits Hold Down Rods** HFT Model **Column Type** Grade Diameter(in) Length (in) D9 & D9H HFTK-D9 STD 32 D12 HFTK-D12 3/4 D18 & D18H HFTK-D18 HS 36
- 1. STD (Standard) rods are ASTM F1554 Grade 36
- 2. HS (High Strength) rods are ASTM A 193 Grade B7
- 3. All Template Kits include:
  - 2-Templates (HFT)
  - · 2-Bolt Braces (HFBB)
  - . 8-Threaded Rods with 2 washers & 4 nuts for each rod

			<b>Hardy Frame Mom</b>	ent Frame: Emb	ed Schedule <sup>1,</sup>	2		
Column Type		Embed From Top of Footing le (in)	Min Edge & End Dist at Footing (in)	Min Edge Dist at T.O. Conc (IN)	Min End Dist at T.O. Conc (IN)	Anchor QTY DIA & Grade Per Column (IN)	Closed Shear Tie Min QTY, Size & Spacing	
D9	D9-Pinned				4			
D9H	D9H-Pinned	12	19	2 3/8	7	4 EA 3/4-STD	4 EA #3 @ 3" OC	
D12	D12-Pinned	1			4-5/8			
D18	D18-Pinned	18	27	2 3/8	7 3/8	4 EA 3/4 - HS	5 EA #3 @ 3" OC	
D18H	D18H-Plinned	10		2 3/0	1 3/0	4 EA 3/4 - N3	5 EA #3 @ 3" UC	

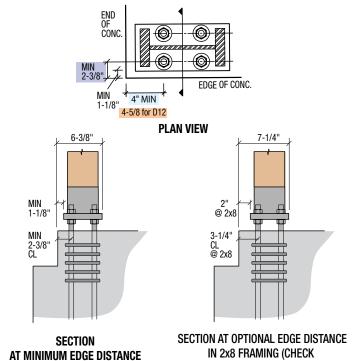
FOUNDATION DESIGN, FOOTINGS AND STEM WALLS SHALL BE DESIGNED BY THE ENGINEER OF RECORD

Notes: 1. Anchors are designed per AISC 341-10 and ACI 318 Appendix D based on  $f'_c$ =2500 psi, fu=60,000 psi and fy=40,000 psi (min)

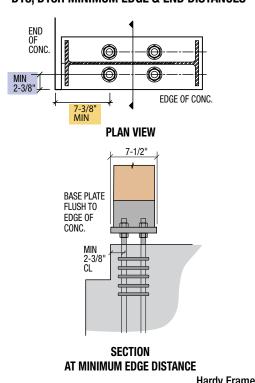
2. For alternate shear transfers or pull-out resistance, calculations shall be supplied by the Engineer of Record

ALIGNMENT OF CONNECTIONS FROM ABOVE)

### D9, D9H & D12 MINIMUM EDGE & END DISTANCES

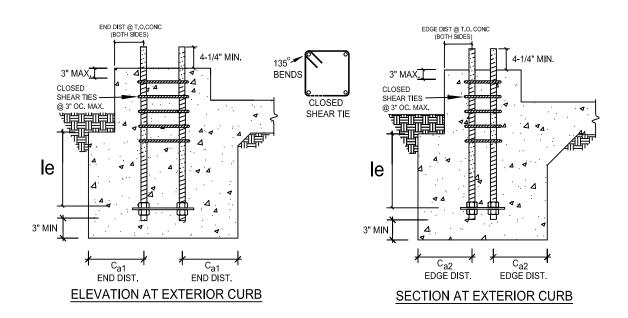


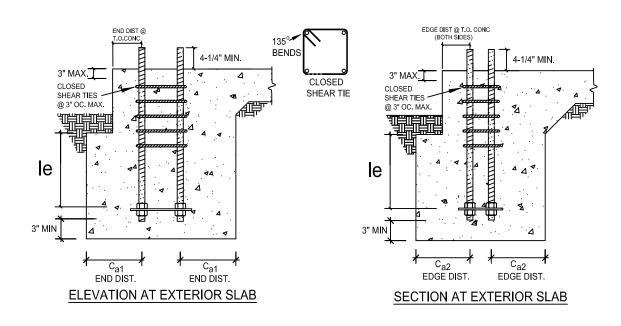
### D18, D18H MINIMUM EDGE & END DISTANCES



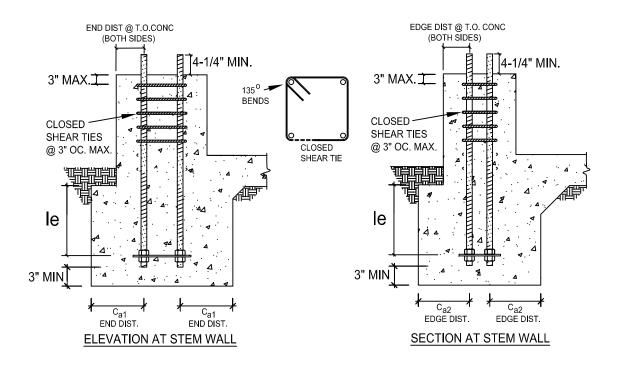


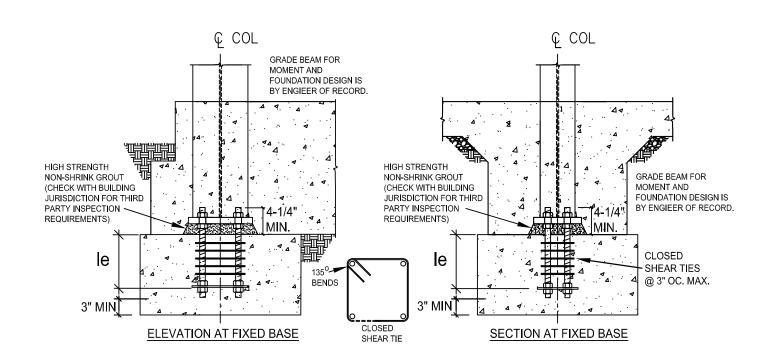
### **Anchorage**





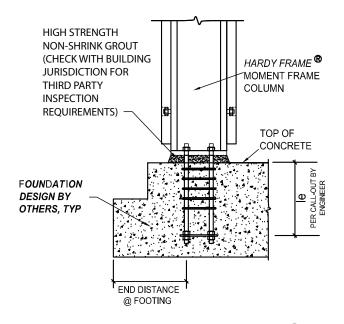
### **Anchorage**





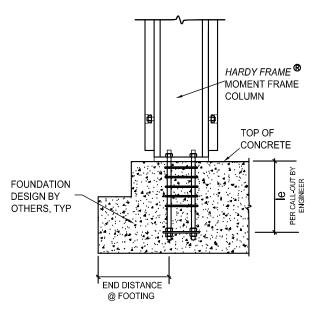
### HARDY A FRAMES.

THE ENGINEER OF RECORD IS PERMITTED TO MODIFY THIS DETAIL TO ACCOMODATE A SPECIFIC CONDITION



ON NUTS AND WASHERS-PINNED BASE ( 1

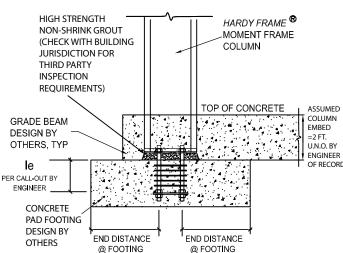
THE ENGINEER OF RECORD IS PERMITTED TO MODIFY THIS DETAIL TO ACCOMODATE A SPECIFIC CONDITION



ON CONCRETE-PINNED BASE (2)

THIS DETAIL IS FOR EXAMPLE ONLY. REFER TO ENGINEER'S CALCS FOR FIXED COLUMN BASE DETAIL.

THIS DETAIL IS FOR EXAMPLE ONLY. REFER TO ENGINEER'S CALCS FOR FIXED COLUMN BASE DETAIL.



ON NUTS AND WASHERS-FIXED BASE (

OF RECORD

PAD FOOTING

**DESIGN BY** 

**OTHERS** 

END DISTANCE

@ FOOTING

HARDY FRAME® MOMENT FRAME COLUMN TOP OF CONCRETE ASSUMED GRADE BEAM COLUMN EMBED DESIGN BY. =2 FT. OTHERS, TYP U.N.O. BY ENGINEER OF RECORD le PER CALL-OUT BY **ENGINEER** CONCRETE.

END DISTANCE

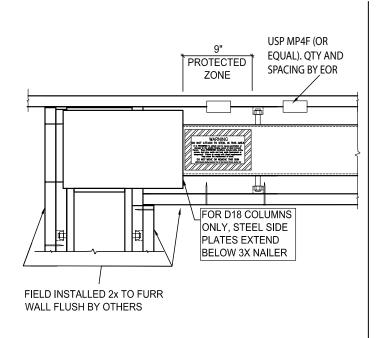
@ FOOTING

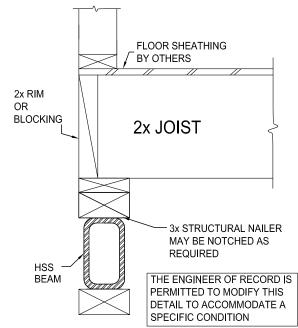
ON CONCRETE-FIXED BASE



### HARDY A FRAMES





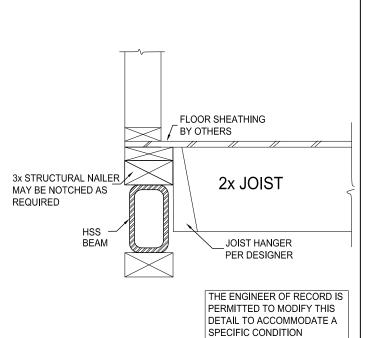


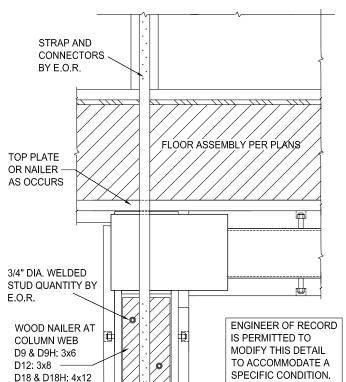
### **ELEVATION WITH 2X TOP PLATE** (



### SECTION @ HEADER - BOTTOM CORD BEARING JOIST







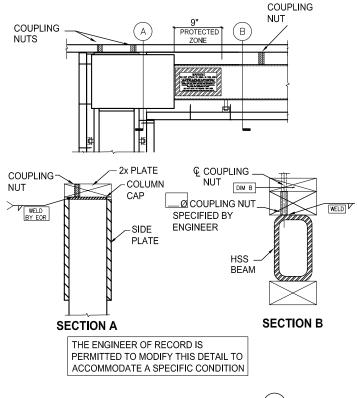
SECTION @ HEADER WITH JOIST HANGING (7

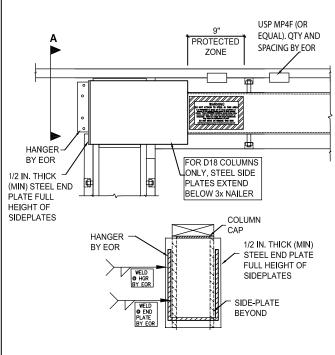


HOLD DOWN STRAP TO COLUMN WEB



### HARDY AT FRAMES



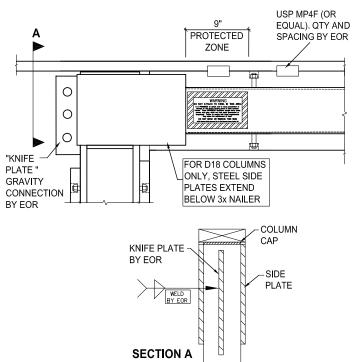


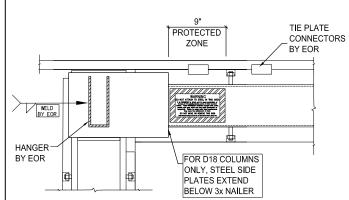
WELDED COUPLING NUTS



HANGER @ COLUMN FLANGE







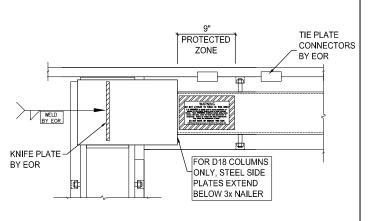
STEEL BEAM TO COLUMN FLANGE (11)

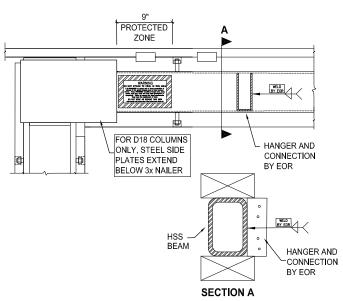
HANGER @ SIDEPLATE FACE



### HARDY AT FRAMES





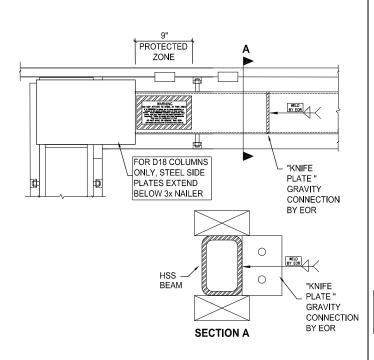


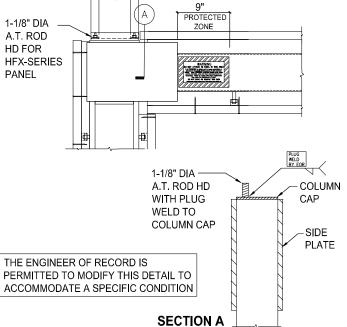
### KNIFE PLATE TO SIDEPLATE FACE



### HANGER TO HEADER







KNIFE PLATE TO HEADER

(15)

PANEL WITH ANCHORS ATTACHED TO MOMET FRAME BELOW 16

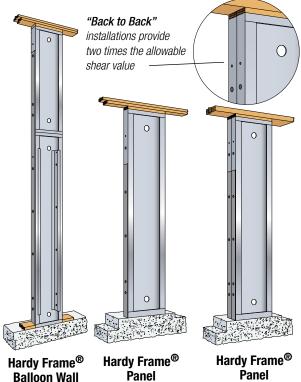
### Hardy Frame® Panels, Brace Frames and Accessories

### **General Information**

Hardy Frames has been leading the pre-manufactured shear wall industry from its beginning. We were the first to be recognized by ICBO-ES and LA City, first to gain approval for multi-story applications, first Balloon Wall application and first to be recognized to comply with the 2003 and 2006 IBC and IRC Building Codes. Today we are the first, and only, to offer a 9 inch Panel width and a Balloon Wall application that is fully assembled in the manufacturing plant and ships as a one piece unit.

All Hardy Frame® Panels and Brace Frames are code listed in ICC-ES ESR-2089 under the 2006 and 2009 IBC Codes and include installations on

concrete, raised floor and upper floor systems.



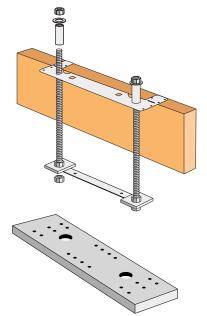
## Hardy Frame® HFX-Series Panels

- Panels are available in 9, 12, 18 and 24 inch widths
- Standard Heights range from 78 inches for portal applications to 20 feet for Balloon Walls
- Custom heights are manufactured routinely
- R Value for design = 6.5, Cd = 4.0
- With proper detailing and anchorage "Back to Back' installations provide two times the allowable shear value without increasing the wall width

### Hardy Frame® HFX-Series Brace Frames

- Brace Frames are available in 32 and 44 inch widths
- Standard Heights range from nominal 8 to 13 feet
- Custom heights are manufactured routinely
- R Value for design = 6.5, Cd = 4.0
- For a given shear load, installing a wider shear wall results in reduced overturning





### **Template Kit (HFXTK)**

Includes:

**Back to Back** 

- One Template for accurate anchor spacing and alignment
- One Bolt Brace for installation at embed end of anchors to prevent independent sway while pouring concrete
- Two 1-1/8 inch diameter hold down anchors
- Six Grade 8 hex nuts
- Two hardened round washers
- Two 1/2 x 3 x 3 plate washers are included when high strength (HS) anchors are specified

### **Hardy Frame Bearing Plate (HFXBP)**

- 3/4" thick x 3 1/2" wide ASTM A36 steel
- Length extends 3" beyond Panel edges. Check for outside corner conditions!
- Reduces wood deformation from overturning forces
- Reduces effects of shrinkage by eliminating bottom plate
   Note: The allowable values in raised floor and upper floor tables assume installation of HFXBP.
   Installation without a HFXBP may result in a reduction of allowable loads.

## Hardy Frame® Moment Frame Non-Standard Worksheet (Please complete one form for each application and fax to Hardy Frame, Inc. 805.477.0993)

Date:		P₁ ↓	
Contact Name:	V 1—		
Company: Phone:  Fax:  Email:		W <sub>IN, MIN</sub>	h <sub>1.MIN</sub>
Address:			
Project:			
City:	<b>←</b>	W <sub>OUT, MAX</sub>	<b></b>
Building Code:	W <sub>IN, I</sub>	<sub>ИІN</sub> h	1, MIN
R:Cd :	VVOUT	r, MAX F	-LIII
Quantity:	V 2—	ļ <sup>2</sup>	
Single Story In Plane Design Shear Required:  V <sub>1</sub> (ASD)lbs.  P <sub>1 DL</sub> lbs.		W <sub>IN, MIN</sub>	h <sub>2.mln</sub>
P <sub>1 snow</sub> lbs. P <sub>1 seismic</sub> lbs.  Two Story In Plane Design Shear Required (as applicable):	V 1→	TOP OF FLOOR SHEATHING	F <sub>2</sub>
V <sub>2</sub> (ASD)lbs.			<b>     </b>
$\begin{array}{cccccccccccccccccccccccccccccccccccc$		W <sub>IN, MIN</sub>	h <sub>1,MIN</sub>
Notes and addtional information		W <sub>OUT, MAX</sub>	
	W <sub>IN, MIN</sub>	h <sub>1, MIN</sub> PL HT <sub>1</sub>	h <sub>2, MIN</sub> PL HT <sub>2</sub>

F<sub>2</sub>\_\_\_\_\_

### **Available Tools From Hardy Frames, Inc.**

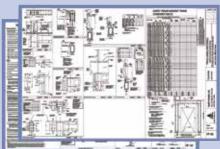
### **CD-ROM**

CD-ROM information includes Code Reports, Typical Installation Details, Tables and Catalogs for all Hardy Frame Products.



### **Typical Installation Detail Pages**

Hardy Frames, Inc. provides Typical Installation Sheets in plan format. These sheets are available in AutoCAD and pdf files, or you may request a hard copy. The information provided may be submitted with building plans as supplemental pages or specific details can be incorporated into the plan set as needed.



### Hardy Frame<sup>®</sup> Moment Frame Calculation Package

For all designs Hardy Frames, Inc. provides a complete Hardy Frame Moment Frame Calculation Package that is stamped and signed by a registered Professional Engineer in the State of California. The package covers the Frame, moment connections, structural wood nailer, anchor rods and base plates.



# HARDY TE FRAMES Inc. A MiTek Company

1732 Palma Dr., Suite 200 Ventura, California 93003 800 754-3030

www.hardyframe.com